



BETTER SOLUTIONS, INTELLIGENTLY ENGINEERED

ENVIRONMENT

Manchester Metropolitan University
Ryebank Road
Chorlton
Flood Risk Statement and Conceptual
Drainage Strategy

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Chorlton
Flood Risk Statement and Conceptual Drainage Strategy

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June 2020

DOCUMENT ISSUE RECORD

Document Number:	MMU-BWB-ZZ-XX-RP-YE-0001_FRA
BWB Reference:	MCW-2136-FRA

Revision	Date of Issue	Status	Author:	Checked:	Approved:
P01	13.03.2020	S2	Lucy Reeves BSc (Hons)	Keith Alger BSc (Hons) MSc	Claire Gardner BSc MSc MCIWEM C.WEM
P02	31.03.2020	S2	Lucy Reeves BSc (Hons)	Keith Alger BSc (Hons) MSc	Claire Gardner BSc MSc MCIWEM C.WEM
P03	30.06.2020	S2	Lucy Reeves BSc (Hons)	Keith Alger BSc (Hons) MSc	Claire Gardner BSc MSc MCIWEM C.WEM

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EXECUTIVE SUMMARY

This Flood Risk Assessment and Conceptual Drainage Strategy has been prepared on behalf of Manchester Metropolitan University to support divestment of their asset at Ryebank Fields, Chorlton (approximate grid reference: SJ810945). The report is therefore not for the purposes of a planning application.

This report demonstrates that the proposed development is at not at a significant flood risk, subject to the flood mitigation strategies being implemented at the planning stage. Any future planning application at the site will require a site-specific Flood Risk Assessment (FRA) and Sustainable Drainage Statement (SDS) to be produced.

The site is shown to be entirely located within Flood Zone 1 (Low Probability). The nearest Flood Zone extents are located approximately 390m south west of the site, associated with the River Mersey and Chorlton Platt Gore. Environment Agency data shows the site to be elevated approximately 4.03m above the 1 in 100-year event and 3.92m AOD in the 1 in 100-year + 30% climate change event flood levels, therefore fluvial flood risk is considered to be low.

A drainage ditch is shown to run through the centre of the site from east to west and is thought to be culverted to the west of the site. A pluvial flow route is shown to run through the site, attributed to the ditch. However, existing levels prevent water from flowing through the ditch and the ditch is therefore not considered to be an active drainage feature.

The site is considered to be at a medium risk of groundwater flooding, and it is recommended that finished floor levels of any buildings are suitably raised above surrounding ground levels to mitigate the groundwater flood risk identified. There is also potential for groundwater to be encountered during construction. Where significant groundwater is encountered within excavations alternative dewatering systems will need to be employed. The advice of a suitably experienced groundwater contractor should be sought to determine the most viable option.

The proposed development has also been assessed against a further range of potential risk sources including surface water, canals, reservoirs and sewers. The site is considered to be at a low risk from these sources.

To mitigate the developments impact on the current runoff regime it is proposed to appropriately manage surface water and foul water separately, in order to ensure flood risk in the wider area is not increased. At this stage it is proposed to store storm water runoff within above ground detention basins and discharge surface water from the site to the existing surface water sewer running through the site. United Utilities have confirmed that foul water will be allowed to drain to the public combined sewer network, with the preferred connection to be to the existing 225mm public combined sewer in Ryebank Road, to the north of the site.

CONTENTS

EXECUTIVE SUMMARY	III
1. INTRODUCTION.....	1
Sources of Data	1
Existing Site.....	2
Proposed Development	3
2. LOCAL GUIDANCE.....	4
Strategic Flood Risk Assessment	4
Preliminary Flood Risk Assessment.....	4
Local Flood Risk Management Strategy.....	4
Greater Manchester Strategic Flood Risk Management Framework	5
Development Framework.....	5
3. POTENTIAL SOURCES OF FLOOD RISK	6
Fluvial Flood Risk.....	7
Coastal/Tidal Flood Risk	11
Flood Risk from Canals	12
Groundwater Flood Risk.....	12
Flood Risk from Reservoirs & Large Waterbodies	13
Pluvial Flood Risk	15
Flood Risk from Sewers	16
Effect of Development on Wider Catchment	17
4. DRAINAGE	18
Surface Water	18
Foul Water.....	21
5. RECOMMENDATIONS AND FURTHER WORKS	22

FIGURES

Figure 1.1: Site Location

Figure 3.1: Flood Map for Planning

Figure 3.2: Ditch, October 2019, Taken Facing East

Figure 3.3: Ditch, November 2019, Taken Facing East

Figure 3.4: Ditch, March 2020, Taken Facing East

Figure 3.5: Culvert, Taken Facing West

Figure 3.6: Reservoir Failure Flood Risk Map

Figure 3.7: Surface Water Flood Risk Map

Figure 3.8: Approximate Location of United Utilities Sewerage Assets (taken from United Utilities Sewer Records and Drain Alert Site Investigations)

TABLES

Table 1.1: Site Summary

Table 3.1: Pre-Mitigation Sources of Flood Risk

Table 4.1: Exiting Greenfield Runoff Rates from the Site

Table 4.2: Restricted Discharge Rate per Catchment

Table 4.3: Total Attenuated Storage Requirement per Catchment

APPENDICES

APPENDIX 1: Topographic Survey

APPENDIX 2: Development Framework

APPENDIX 3: EA Correspondence

APPENDIX 4: Manchester City Council Correspondence

APPENDIX 5: Sewer Records

APPENDIX 6: Drain Alert Report

APPENDIX 7: Micro Drainage Calculations

APPENDIX 8: Conceptual Drainage Strategy

APPENDIX 9: Pre-Development Enquiry

1. INTRODUCTION

- 1.1 This Flood Risk Assessment (FRA) and Conceptual Drainage Strategy has been prepared on behalf of Manchester Metropolitan University to support divestment of their asset at Ryebank Fields, Chorlton.
- 1.2 A Development Framework document has been produced on behalf of Manchester Metropolitan University to inform future proposals. The framework has been endorsed by Manchester City Council's Executive, to guide and assist the assessment of future planning applications for the site.
- 1.3 This FRA and Conceptual Drainage Strategy is not intended to support a planning application and as such the level of detail included is commensurate and subject to the nature of the proposals.

Table 1.1: Site Summary

Site Name	Ryebank Road
Location	Chorlton
NGR (approx.)	SJ810945
Application Site Area (ha)	4.68 (approx.)
Development Type	Residential
Flood Zone Classification	Flood Zone 1
NPPF Vulnerability	More Vulnerable
Environment Agency Office	Greater Manchester, Merseyside and Cheshire
Lead Local Flood Authority	Manchester City Council
Local Planning Authority	Manchester City Council

Sources of Data

- i. Topographical Survey by CT Surveys, reference [Dwg No. 4692AB/1-5]
- ii. OS Explorer Series mapping
- iii. Environment Agency consultation
- iv. Manchester City Council Consultation
- v. Local Authority Surface Water Flood Risk Maps
- vi. Ryebank Road Development Framework document
- vii. Manchester City, Salford City and Trafford Council Strategic Flood Risk Assessment
- viii. Manchester City Council Preliminary Flood Risk Assessment

- ix. Site visit undertaken by BWB Consulting Ltd
- x. Manchester City Council Local Flood Risk Management Strategy
- xi. Ground Investigations undertaken by e3p, reference [13-355-R2-DRAFT and 13-533-R1-1]
- xii. CCTV Survey undertaken by Drain Alert Ltd
- xiii. Utility Assessment Report undertaken by Fuel Solutions UK Limited
- xiv. United Utilities Sewer Records
- xv. United Utilities Pre- Development Enquiry
- xvi. British Geological Survey Drift & Geology Maps

Existing Site

- 1.4 The site is located on the land of Ryebank Fields, Ryebank Road, Chorlton, approximately 4.3km south west of Manchester. The site is bound to the north and east by residential development, to the south by Longford Road and to the west by Longford Park. The existing site comprises open space with an area of hardstanding to the south of the site. The site's location is illustrated within **Figure 1.1**.



Figure 1.1: Site Location

- 1.5 A topographic survey has been undertaken and is included as **Appendix 1**. The site is shown to be relatively flat with a gentle fall in a north westerly direction. Site levels are shown to range from approximately 26.20m Above Ordnance Datum (AOD) in the north west to 28.62m AOD in the south. A ditch is shown to run through the centre of the site in a westerly direction with levels ranging from 27.11m AOD at the top of the banks to 26.20m AOD at the bottom of the banks. Parts of the site have not been surveyed due to densely overgrown vegetation.

Proposed Development

- 1.6 It is expected that the proposed development will be residential, split into two parcels in the north and south, comprising approximately 120 properties and including detached, semi-detached and terraced properties along with associated open space, as informed by the Development Framework. An extract of the Development Framework is included as **Appendix 2**. Proposed plans are not available at the time of writing.

2. LOCAL GUIDANCE

Strategic Flood Risk Assessment

- 2.1 A Strategic Flood Risk Assessment (SFRA) is a study carried out by one or more local planning authorities to assess the risk to an area from flooding from all sources, now and in the future.
- 2.2 The Manchester City, Salford City and Trafford Councils Level 1 SFRA¹ has been reviewed in the production of this FRA. The SFRA provides information specific to the site location in the form of fluvial, surface water and groundwater flood risk mapping, as well as records of historic flooding. Information from the Level 1 SFRA will be referenced within **Section 3.0** where applicable.
- 2.3 The Manchester City, Salford City and Trafford Councils Level 2 SFRA² was produced to facilitate the application of Sequential and Exception Tests to screen allocated development sites. The proposed application site is not referenced within the Level 2 SFRA. Information from the Level 2 SFRA will be referenced within **Section 3.0** where applicable.

Preliminary Flood Risk Assessment

- 2.4 A Preliminary Flood Risk Assessment (PFRA) is an assessment of floods that have taken place in the past and floods that could take place in the future. It generally considers flooding from surface water runoff, groundwater and ordinary watercourses, and is prepared by the Lead Local Flood Authorities.
- 2.5 The Manchester City Council PFRA³ considers flooding from surface water runoff, groundwater, ordinary watercourses and canals. However, no historical instances of flooding at the site are referenced. Information from the PFRA will be referenced within this report where applicable.

Local Flood Risk Management Strategy

- 2.6 A Local Flood Risk Management Strategy (LFRMS) is prepared by a Lead Local Flood Authority to help understand and manage flood risk at a local level. The LFRMS aims to ensure that the knowledge of local flood risk issues is communicated effectively so that they can be better managed. The LFRMS also aims to promote sustainable development and environmental protection.
- 2.7 The Manchester City Council LFRMS⁴ has been reviewed and will be referenced within this report where applicable.

¹ Level 1 Strategic Flood Risk Assessment (Manchester City, Salford City and Trafford Council, 2010)

² Level 2 Strategic Flood Risk Assessment (Manchester City, Salford City and Trafford Council, 2011)

³ Preliminary Flood Risk Assessment (Manchester City Council, 2011)

⁴ Local Flood Risk Management Strategy (Manchester City Council, 2014)

Greater Manchester Strategic Flood Risk Management Framework

- 2.8 The Greater Manchester Strategic Flood Risk Management Framework (SFRMF)⁵ aims to provide a spatial framework for flood risk management that highlights the key strategic flood risks including cross-boundary issues and recommends key priorities for intervention.
- 2.9 The Greater Manchester SFRMF has been reviewed and will be referenced within the report where applicable.

Development Framework

- 2.10 A Development Framework⁶ has been prepared on behalf of Manchester Metropolitan University for Ryebank Road, Chorlton, Manchester. The role of the framework is to define the broad parameters that future development can be set within. It does not provide detailed design of the development project; this is something that will be dealt with through individual planning applications.
- 2.11 The Framework aims to provide the conditions to bring forward a high quality residential neighbourhood in keeping with its surrounding context whilst supporting a new residential product that will enhance the existing residential offer of Chorlton and the wider South Manchester market.

⁵ Greater Manchester Strategic Flood Risk Management Framework (Greater Manchester Combined Authority, 2018)

⁶ Development Framework, Manchester Metropolitan University (2019)

3. POTENTIAL SOURCES OF FLOOD RISK

3.1 Flooding can occur from a variety of sources, or combination of sources, which may be natural or artificial. **Table 3.1** below identifies the potential sources of flood risk to the site in its current condition, and the impacts which the development could have in the wider catchment, prior to mitigation. These are discussed in greater detail in the forthcoming section. The mitigation measures proposed to address flood risk issues and ensure the development is appropriate for its location are discussed within **Section 4.0**.

Table 3.1: Pre-Mitigation Sources of Flood Risk

Flood Source	Potential Risk				Description
	High	Medium	Low	None	
Fluvial			X		The site is entirely located in Flood Zone 1. Longford Brook is located approximately 150m north west of the site.
Coastal				X	The site is shown to be located away from any coastal/tidal influence.
Canals			X		The site is shown to be located away from the Bridgewater Canal.
Groundwater		X			The site is shown to fall within an area predicted to be at a medium susceptibility to groundwater flooding.
Reservoirs and waterbodies			X		The site is shown to fall within the area at risk of reservoir failure. However, maintenance and safety checks mean this only poses a low residual risk to the site.
Pluvial runoff			X		The site is shown to be largely at a very low risk of pluvial flooding with a pluvial flow route shown to run through the centre of the site, associated with the existing ditch.
Sewers			X		The surface water sewer running through the site is of sufficient depth that it is not thought to pose a flood risk.
Effect of Development			X		Development will not result in impedance/loss of surface water route.

Flood Source	Potential Risk				Description
	High	Medium	Low	None	
on Wider Catchment		X			The development will increase the area of impermeable surfaces leading to a potential increase in runoff.

Fluvial Flood Risk

- 3.2 Flooding from watercourses occurs when flows exceed the capacity of the channel, or where a restrictive structure is encountered, which leads to water overtopping the banks into the floodplain. This process can be exacerbated when debris is mobilised by high flows and accumulates at structures.
- 3.3 The site is shown to be located entirely within Flood Zone 1, as shown in **Figure 3.1**. The nearest Environment Agency (EA) Main River is Longford Brook, located approximately 150m north west of the site in Longford Park and flows in a north westerly direction. The watercourse is assumed to be culverted with no Flood Zones associated with it within the vicinity of the site.



Figure 3.1: Flood Map for Planning

- 3.4 The nearest Flood Zone 2 and 3 extents are located approximately 390m south west of the site, associated with the River Mersey and Chorlton Platt Gore.
- 3.5 The EA were contacted to request flood information available at the site and surrounding area. The correspondence is included as **Appendix 3**. The Chorlton Platt Gore 2012 model data provided by the EA shows the modelled water level associated with Chorlton Platt Gore to be 24.59m AOD during the 1 in 100-year undefended and defended event at the nearest modelled node, located approximately 1km south east of the site. The modelled water level during the 1 in 100-year + 30% climate change event is shown to be 24.70m AOD in both the undefended and defended scenarios. The site is shown to be elevated approximately 4.03m above the 1 in 100-year event and 3.92m AOD in the 1 in 100-year + 30% climate change event.
- 3.6 The EA Historical Flood Map shows the nearest recorded flood outline to be located approximately 2.5km south east of the site, associated with a 1964 flood event. Correspondence with Manchester City Council as the LLFA (included as **Appendix 4**) notes no record of any flooding at the site or within the immediate vicinity of the site.

- 3.7 Based upon the distance and intervening topography, the site is considered to be at a low risk from fluvial sources.

Longford Brook/Nico Ditch

- 3.8 A drainage ditch is shown to pass through the centre of the site from east to west and is currently understood to serve a small catchment of approximately 1.4km. Historical mapping (referenced within the Phase I Geoenvironmental Site Assessment⁷) suggests that Longford Brook once passed through the site to the north of the existing ditch before being diverted along the eastern site boundary.
- 3.9 A site visit undertaken by BWB Consulting in October 2019 noted that the ditch is not well defined with large amounts of vegetation and no apparent flowing water, as shown in **Figure 3.2**. A site visit undertaken in November 2019, following vegetation clearance, also shows the ditch to be dry, with no flowing water, as shown in **Figure 3.3**. A further site visit, undertaken in March 2020, noted standing water within the ditch, as shown in **Figure 3.4**. The ditch is, therefore, not considered to be an active drainage feature due to the lack of flowing water within the feature.



Figure 3.2: Ditch, October 2019, Taken Facing East

⁷ Phase I Geoenvironmental Site Assessment, e3p (2020)



Figure 3.3: Ditch, November 2019, Taken Facing East



Figure 3.4: Ditch, March 2020, Taken Facing East

- 3.10 It is considered at this stage that there is no connectivity between the eastern boundary diversion of the Longford Brook and the drainage ditch, as the ditch was noted as not conveying any flows during a period of intense rainfall.
- 3.11 The ditch is understood to be culverted to the western boundary of the site and Longford Park, as shown in **Figure 3.5**. Standing water is shown to be present at the entrance to the culvert, implying that the culvert may be blocked, preventing the flow of water away from the site. Further investigation of the culvert is required in order to understand the connectivity of the ditch to the west of the site and confirm any connectivity to the Longford Brook. Recommendations for further investigation is included within **Section 5.0**.



Figure 3.5: Culvert, Taken Facing West

- 3.12 Overall, the fluvial flood risk surrounding the existing ditch is considered to be low.

Coastal/Tidal Flood Risk

- 3.13 Inundation of low lying coastal areas by the sea may be caused by seasonal high tides, storm surges and storm driven wave action. Coastal/Tidal flooding is most commonly a result of a combination of two or more of these mechanisms, which can result in the overtopping or breaching of sea defences. River systems may also be subject to tidal influences.
- 3.14 The site is shown to be located away from any coastal/tidal influence. Therefore, there is no risk of coastal/tidal flooding at the site.

Flood Risk from Canals

- 3.15 The Canal and River Trust (CRT) generally maintains canal levels using reservoirs, feeders and boreholes and manages water levels by transferring it within the canal system.
- 3.16 Water in a canal is typically maintained at predetermined levels by control weirs. When rainfall or other water enters the canal, the water level rises and flows out over the weir. If the level continues rising it will reach the level of the storm weirs. The control weirs and storm weirs are normally designed to take the water that legally enters the canal under normal conditions. However, it is possible for unexpected water to enter the canal or for the weirs to become obstructed. In such instances the increased water levels could result in water overtopping the towpath and flowing onto the surrounding land.
- 3.17 Flooding can also occur where a canal is impounded above surrounding ground levels and the retaining structure fails.
- 3.18 The Bridgewater Canal is located approximately 1km west of the site. The LFRMS identifies the site to be located outside of the canal breach zone and the SFRA notes limited canal overtopping in Manchester with overtopping events unlikely to affect areas beyond the canal towpath. Therefore, the flood risk associated with canals is thought to be low.

Groundwater Flood Risk

- 3.19 Groundwater flooding occurs when the water table rises above ground elevations. It is most likely to happen in low lying areas underlain by permeable geology. This may be regional scale chalk or sandstone aquifers, or localised deposits of sands and gravels underlain by less permeable strata such as that in a river valley.
- 3.20 According to British Geological Survey (BGS) mapping, the area is shown to be underlain by the Wilmslow Sandstone Formation, which is designated as a Principal Aquifer. Principal Aquifers are layers of rock or drift deposits that have high intergranular and/or fracture permeability, meaning they usually provide a high level of water storage. They may support water supply and/or base flow on a strategic scale.
- 3.21 Superficial deposits of Till (Devensian) are expected to be present throughout the majority of the site with a small portion to the north western corner of the site expected to be underlain by Glaciofluvial Sheet Deposits (Devensian). The superficial deposits are designated Secondary (undifferentiated) and Secondary A Aquifers.
- i. Secondary (undifferentiated) Aquifers are assigned in cases where it has not been possible to attribute either Category A or B to a rock type.
 - ii. Secondary A Aquifers are permeable layers capable of supporting water supplies at a local rather than strategic scale, and in some cases forming an important source of base flow to rivers.
- 3.22 Ground investigations carried out by e3p in December 2019 encountered Made Ground deposits in all exploratory hole locations to a maximum depth of 12.70m Below Ground Level (BGL).

- 3.23 Site investigations including 30 trial pits and 16 borehole logs have been undertaken by e3p. Groundwater was encountered within 24 trial pits between depths of 1.4m BGL and 2.9m BGL. Several of the trial pits had to be terminated due to rapid groundwater ingress, particularly within the south of the site. Groundwater was encountered within 8 borehole logs between depths of 2.1m BGL and 4.3m BGL. Further information can be found within the Phase II Geoenvironmental Site Assessment⁸.
- 3.24 The Manchester City Council PFRA notes that the site is located within an area at risk of groundwater rebound and the LFRMS shows the site to be located within an area at risk of groundwater flooding; however, the SFRA notes no known records of groundwater flooding in Manchester and correspondence with the LLFA notes no record of flooding within the site.
- 3.25 A site visit undertaken by BWB Consulting in October 2019 noted muddy ground conditions, but no standing water appeared to be present. A further site visit in March 2020 also noted muddy ground conditions but with standing water pooling in the rutted areas, following a period of rainfall.
- 3.26 The proposed development is considered to be at a medium risk of groundwater flooding.
- 3.27 In the event of groundwater flooding, due to the relatively flat nature of the site, water is likely to pool across the site. Additionally, whilst there is scope for groundwater to rise through such deposits, in some cases, impermeable surfaces at the site would act as a barrier to groundwater emergence.
- 3.28 Mitigation measures are required to reduce the potential risk of groundwater flooding, especially during construction. Further details on which are included within **Section 5.0**.

Flood Risk from Reservoirs & Large Waterbodies

- 3.29 Flooding can occur from large waterbodies or reservoirs if they are impounded above the surrounding ground levels or are used to retain water in times of flood. Although unlikely, reservoirs and large waterbodies could overtop or breach leading to rapid inundation of the downstream floodplain.
- 3.30 To help identify this risk, reservoir failure flood risk mapping has been prepared, this shows the largest area that might be flooded if a reservoir were to fail and release the water it holds. The map displays a worst case scenario and is only intended as a guide. An extract from the mapping is included as **Figure 3.6**.

⁸ Phase II Geoenvironmental Site Assessment, e3p (2020)



Figure 3.6: Reservoir Failure Flood Risk Map

- 3.31 The site is shown to be located within the potential failure flood extent of up to four reservoirs, this could pose a hazard to the site in its existing conditions.
- 3.32 The EA data (included as **Appendix 3**) confirmed the residual risk of reservoir flooding to the site. The reservoirs are owned and operated by United Utilities Water plc, who have the ultimate responsibility for the safety of their reservoir assets. Their responsibilities include regular safety inspections, any necessary design or repairs undertaken where required and an annual statement produce on the operation and maintenance regime.
- 3.33 Based on the safety legislation in place and the maintenance and repair responsibilities of the reservoir owners, the actual probability of a significant failure is considered to be low. Therefore, the risk of flooding at the site from this source is also considered to be low.

Pluvial Flood Risk

- 3.34 Pluvial flooding can occur during prolonged or intense storm events when the infiltration potential of soils, or the capacity of drainage infrastructure is overwhelmed leading to the accumulation of surface water and the generation of overland flow routes.
- 3.35 Risk of flooding from surface water mapping has been prepared, this shows the potential flooding which could occur when rainwater does not drain away through the normal drainage systems or soak into the ground but lies on or flows over the ground instead. An extract from the mapping is included as **Figure 3.7**.

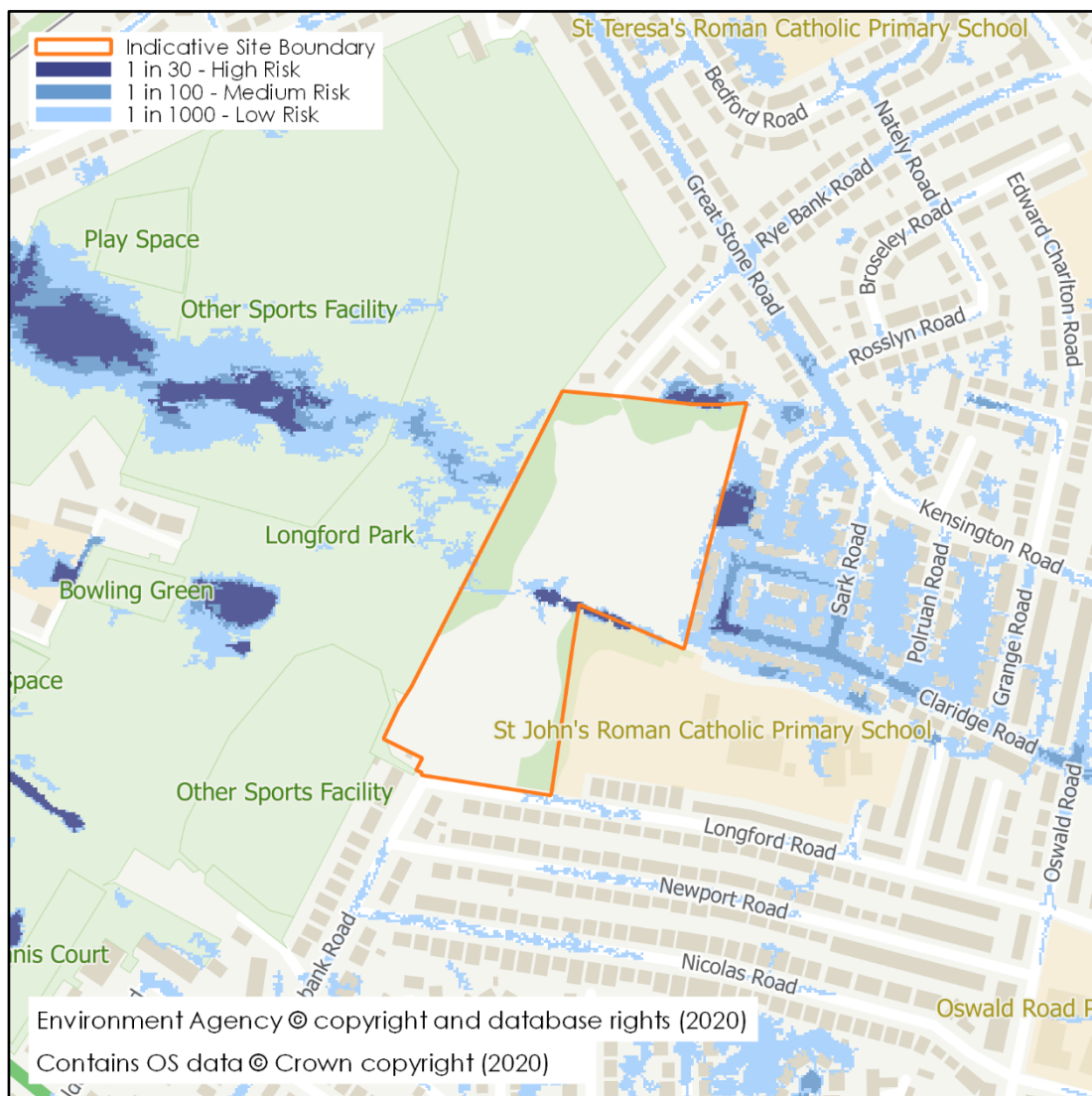


Figure 3.7: Surface Water Flood Risk Map

- 3.36 The site is largely shown to be at a very low risk of surface water flooding. A pluvial flow route is shown to flow through the centre of the site from east to west presenting a Low (1 in 1000-year) to High (1 in 30-year) risk of surface water flooding. This is thought to be associated with the ditch on site and is shown to be generally constrained. Due to existing levels of the ditch, water cannot flow and collects as standing water as a result of prolonged rainfall, as shown by the small areas of high susceptibility.

- 3.37 A large area of low to high surface water flood risk is shown to be present to the north east and east of the site. To the south of the site, Ryebank Road and Longford Road are largely shown to be at a very low risk of surface water flooding, therefore safe/dry access and egress is achievable.
- 3.38 The Greater Manchester SFRMF identifies surface water flood risk as a general issue for Greater Manchester, however it does not provide details on the exact areas deemed to be affected.
- 3.39 The overall flood risk to the site from pluvial sources is thought to be low.

Flood Risk from Sewers

- 3.40 Sewer flooding can occur when the capacity of the infrastructure is exceeded by excessive flows, or as a result of a reduction in capacity due to collapse or blockage, or if the downstream system becomes surcharged. This can lead to the sewers flooding onto the surrounding ground via manholes and gullies, which can generate overland flows.
- 3.41 The local sewerage undertaker is United Utilities Water. A copy of their sewer records is included as **Appendix 5**.
- 3.42 A public surface water sewer with a construction comprising of brick, a diameter of 1275mm and an average depth to invert of approximately 5.40m is shown to be present in the south western corner of the site. An overflow pipe with a diameter of 1500mm is located to the north of the site and is known to flow from the north where it serves a wider area, the depth of the pipe is not known. There are a number of public foul water sewers located to the north, east and south of the site, which serve the existing residential development.
- 3.43 A CCTV survey and investigation of the public surface water sewer were undertaken by Drain Alert (included as **Appendix 6**) in order to understand the connectivity between any sewers on site. Die testing and sounding out proved the connectivity of the surface water sewer in the south west of the site to the overflow sewer to the north of the site. Therefore, the public surface water sewer is shown to run through the entire site as shown in **Figure 3.8**. The location of the sewer is approximate and based on site investigations and existing United Utilities Sewer Records.
- 3.44 There is a need for any future development to consider the sewer by means of an easement. Further details are included within **Section 5.0**.
- 3.45 In the unlikely event of sewer flooding, water will follow local topography and may pool in places due to the relatively flat nature of the topography or flow in a north westerly direction.
- 3.46 Due to the depth of the sewer within the site, flood risk from sewers is considered to be low.

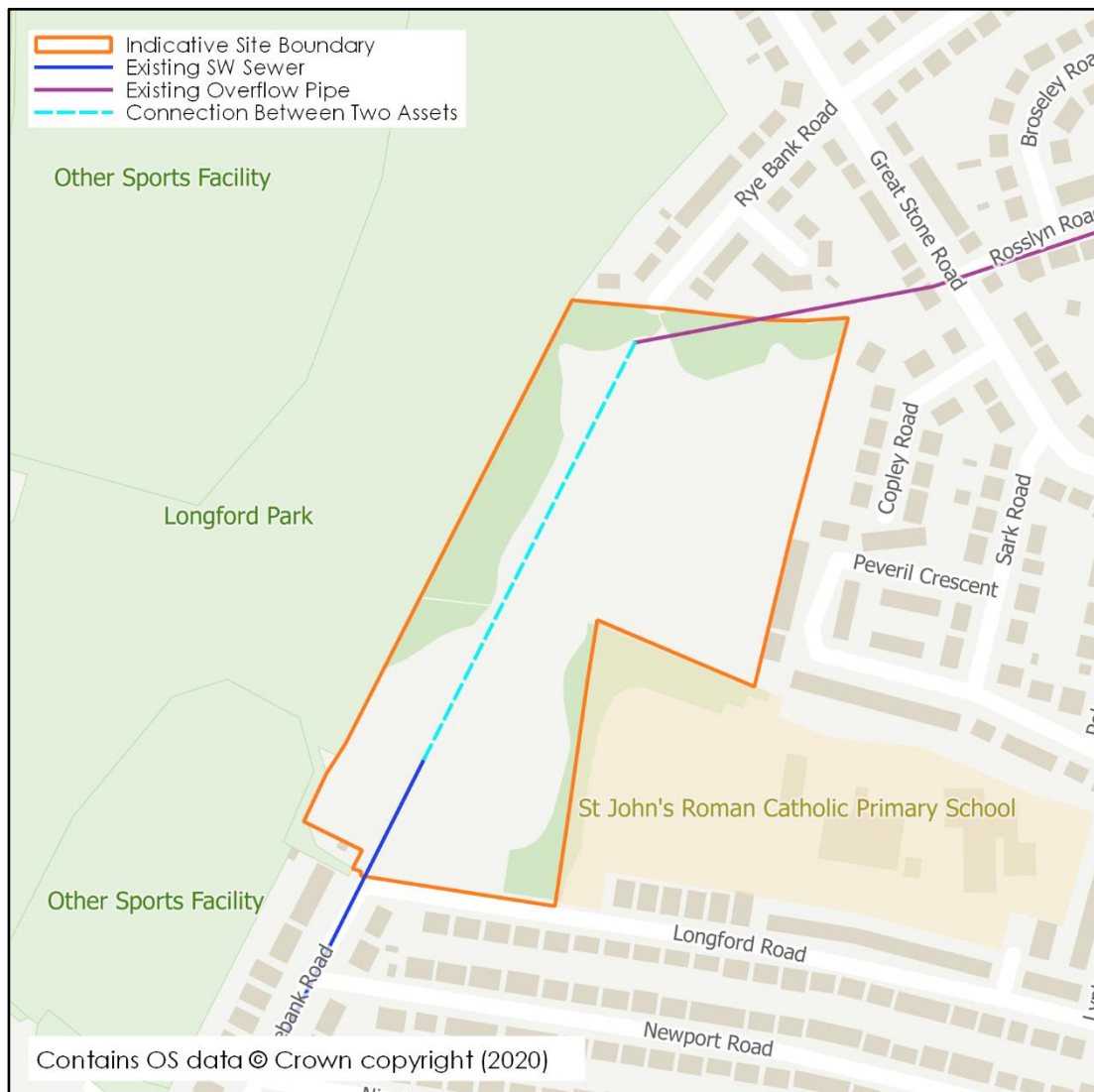


Figure 3.8: Approximate Location of United Utilities Sewerage Assets (taken from United Utilities Sewer Records and Drain Alert Site Investigations)

Effect of Development on Wider Catchment

Impedance of Flood Flows

- 3.47 The Development Framework suggests that development will be split into two parcels in the north and south of the site with green infrastructure located along the western site boundary and along the alignment of the existing ditch. Therefore, development will not impede any pluvial flow routes through the site.

Development Land Use

- 3.48 The proposed development will increase the area of impermeable surfaces on site. This will result in an increase in surface water runoff, which could increase flood risk downstream unless properly mitigated.

4. DRAINAGE

Surface Water

Drainage Requirements

- 4.1 Local and national policy requires the use of Sustainable Drainage Systems (SuDS) principles for new developments, which would necessitate consideration of the infiltration potential of the ground, in the first instance, as a means of surface water disposal, followed by discharge to a local waterbody. An allowance of attenuation and treatment of surface water will also be required.
- 4.2 Manchester City Council, in their role as LLFA, have published a SuDS Requirements for New Developments Update 2019⁹ which advises that SuDS are designed in line with the national Non-Statutory Technical Standards for SuDS¹⁰. This guidance has been used to inform this assessment and is included as **Appendix 4**.
- 4.3 Typically, an allowance for urban creep, to account for residents increasing the impermeable areas through driveways and patios etc, should be included and usually depends on the density of the residential development. Given the early and conceptual stage of this surface water drainage strategy, no allowance has been included for urban creep. Once an indicative masterplan is available, a more detailed strategy that includes an appropriate allowance for urban creep should be produced. Typically, an additional 10% of the proposed impermeable area would be required to accounted for as part of the site drainage.

Existing Conditions

- 4.4 The total site is approximately 4.68ha. The developable area, as measured by the two indicative catchments, is approximately 2.99ha.
- 4.5 The runoff rate, per hectare, for the site has been estimated using the IH124 method, with appropriate prorated adjustments for a site of less than 50ha. This was undertaken within Micro Drainage, which makes the necessary adjustments for small sites automatically. The results are summarised within **Table 4.1** and included as **Appendix 7**.

Table 4.1: Exiting Greenfield Runoff Rates from the Site

Return Period (Yrs.)	Runoff Rate (l/s/ha)
1	2.0
Mean Annual Flow Rate (QBAR)	2.3
30	3.9
100	4.8

⁹ SuDS Requirements for New Developments, Manchester City Council (2019)

¹⁰ Sustainable Drainage Systems Non-statutory technical standards for sustainable drainage systems, DEFRA (2015)

Drainage Hierarchy

- 4.6 The Planning Policy Guidance¹¹ and the SuDS Manual identify that surface water runoff from a development should be disposed of as high up the following hierarchy as reasonably practicable:
- i. Into the ground (infiltration);
 - ii. To a surface water body;
 - iii. To a surface water sewer, highway drain, or another drainage system;
 - iv. To a combined sewer.
- 4.7 In-situ (falling) head permeability tests were undertaken by e3p as part of the Phase II investigations, within two environmental monitoring wells. As the site is underlain by Made Ground impacted by low-level inorganic and hydrocarbon compounds, soakaway drainage is not considered to be suitable for the proposed development.
- 4.8 A drainage ditch is shown to run through the centre of the site with a culvert located to the western site boundary. The ditch is not understood to be an active drainage feature with the existing levels preventing water from flowing within the ditch. At this stage, the ditch is not considered to be a suitable receiving body for surface water runoff.
- 4.9 Due to the presence of the surface water sewer running through the entire site, it is proposed to discharge surface water from the site to this asset following development. This is subject to confirmation from United Utilities.

Surface Water Drainage Strategy

- 4.10 The site has been split into two drainage catchments, with the existing drainage ditch separating the two parcels, in line with the Development Framework.
- 4.11 The Non-statutory technical standards for sustainable drainage systems requires that the peak runoff rate from the development to any highway drain, sewer or surface water body for the 1 in 100-year rainfall event should never exceed the peak greenfield runoff rate from the same event.
- 4.12 The greenfield runoff rate for each catchment has been calculated using the catchment area and the existing greenfield rate of 2.3l/s/ha. The results are shown in **Table 4.2**. The greenfield runoff rate for Catchment 2 has been increased to 3l/s in order to reduce the potential risk of blockage of flow restriction devices, this increased runoff rate has been used to calculate the attenuated storage requirement for the catchment.

¹¹ Planning Practice Guidance. <http://planningguidance.planningportal.gov.uk/>

Table 4.2: Restricted Discharge Rate per Catchment

Catchment No.	Catchment Area (ha)	Greenfield Runoff Rate (l/s)
1	1.95	4.5
2	1.04	2.4

- 4.13 A simulation has been run using Micro Drainage 'Source Control' to identify the necessary storage provision for each catchment. Using a restriction of 4.5l/s for Catchment 1 and 3l/s for Catchment 2, and a proposed impermeable area of 1.27ha for Catchment 1 and 0.68ha for Catchment 2, the volume of attenuated storage required for the development has been calculated for storm events up to the 1 in 100-year + 40% climate change storm. The results are summarised in **Table 4.3** and included as **Appendix 7**.

Table 4.3: Total Attenuated Storage Requirement per Catchment

Catchment No.	Critical Storm	Maximum Volume (m ³)
1	1440 min Winter	1019.4
2	960 min Winter	511.2

- 4.14 The catchment-based approach will treat and attenuate the surface water runoff as close to its source as possible. Storm water runoff from each catchment will be stored within an above ground detention basin. The water will then discharge at a controlled rate via a vortex flow control or similar device, into the receiving surface water network.
- 4.15 Water quality should be considered within the drainage strategy to capture any potential pollutants in the runoff from the development. The detention basins will provide the primary stage of treatment. However, to supply a minimum two stage treatment train, further features such as conveyance swales, bioretention areas and pervious pavements should be incorporated into the development and included within the detailed design. Rainwater harvesting measures should also be considered as a method of recycling surface water and reducing/delaying surface water entering the surrounding network.
- 4.16 As part of any future planning application, this conceptual drainage design should be developed into a more detailed drainage strategy which should be created alongside the masterplan to ensure that a suitable area is designated for SuDS in line with local and national guidance.
- 4.17 A Conceptual Surface Water Drainage Strategy is included as **Appendix 8** (Drawing reference: MMU-BWB-ZZ-XX-SK-CD-0001) and demonstrates how the required storage could be achieved within each catchment.

Foul Water

- 4.18 United Utilities have confirmed via a Pre-development Enquiry (included as **Appendix 9**), that foul will be allowed to drain to the public combined sewer network. Their preferred
- 4.19 connection would be to the existing 225mm public combined sewer in Ryebank Road, to the north of the site.
- 4.20 A formal application will have to be made to United Utilities in order to make a formal sewer connection.

5. RECOMMENDATIONS AND FURTHER WORKS

- 5.1 This report has been produced on behalf of Manchester Metropolitan University to support divestment of their asset at Ryebank Fields, Chorlton (approximate grid reference: SJ810945).
- 5.2 Any future planning application at the site will require a site-specific Flood Risk Assessment (FRA) and Sustainable Drainage Statement (SDS) to be produced.

Flood Risk

- 5.3 The site has been identified to fall within Flood Zone 1 (Low Probability). A drainage ditch is shown to run through the centre of the site from east to west; however, this is not considered to be an active drainage feature due to the lack of flowing water. The ditch is culverted to the west of the site, further investigation of the connectivity of the culvert to the west of the site is recommended.
- 5.4 An overland flow route is shown to be present through the centre of the site, attributed to the drainage ditch. Mitigation would be required to ensure obstructions are not placed over natural overland flow routes.
- 5.5 The site has been identified to be at a medium risk of groundwater flooding. It is recommended that finished floor levels of any buildings are suitably raised above surrounding ground levels to mitigate the groundwater flood risk identified. There is also potential for groundwater to be encountered during construction. Where significant groundwater is encountered within excavations alternative dewatering systems will need to be employed. The advice of a suitably experienced groundwater contractor should be sought to determine the most viable option.
- 5.6 Due to the presence of the surface water sewer passing through the site there is a need for any future development to consider this by means of an easement, where no built development or planting can take place. Initial correspondence from United Utilities would suggest that such a sewer would require an easement of 3m either side of the sewer centre line. However, due to its depth and diameter it is thought there is the potential for an easement of circa 8m either side of the sewer. Further clarification has been sought from United Utilities on this matter, at the time of writing a response is yet to be received.
- 5.7 The site has also been assessed against a range of potential flood risk sources including surface water, sewers, canals and reservoirs. None of these flood sources are thought to represent a potential barrier to development.

Surface Water Drainage

- 5.8 Due to the increase in impermeable area and the resulting implication upon the surface water runoff regime, a Sustainable Drainage Strategy which incorporates SuDS into the development in line with the latest guidance would be required.

- 5.9 It is proposed at this stage to discharge surface water from the site to the existing surface water sewer running through the site. This is subject to confirmation from United Utilities.
- 5.10 A catchment-based approach has been applied and the site has been divided into two catchments based on the Development Framework. This approach aims to treat and attenuate the surface water runoff as close to its source as possible. Storm water runoff from each catchment will be stored within above ground detention basins. The water will then discharge at the equivalent greenfield rate up to the 1 in 100-year storm with a 40% allowance for climate change via a vortex flow control into the receiving surface water network.

Foul Water

- 5.11 United Utilities have confirmed in their Pre-Development Enquiry response that foul will be allowed to drain to the public combined sewer network. Their preferred connection would be to the existing 225mm public combined sewer in Ryebank Road, to the north of the site.
- 5.12 A formal application will have to be made to United Utilities, at the appropriate juncture in order to make a formal sewer connection.

APPENDICES

APPENDIX 1: Topographic Survey

ABBREVIATIONS

AR ASSURED ROUTE	OA OUTSIDE SURVEY AREA
AV AFB VALVE	PF FOOT POLE
B BOLLARD	RL RISE LEVEL
BF BENCH MARK	RF RISE/FALL
BT BENCH TELECOM CHAMBER	RS ROAD SIGN
CA CABLE TV	ROF ROAD SIGN
CC COVER LEVEL	SC STOP COCK
CD CABLE DEPTH	SD STOP SIGN
CL COVER LEVEL	SV STOP VALVE
CP DOWN PIPE	SW SWALE/WEST PIPE
EL ELECTRICITY POLE	TC TRAFFIC CONTROL
EM ELECTRICITY METER	TOB TELEPHONE CABINET
EP ELECTRICITY POLE	TR TRUCK TRAIL RECORDS
FR FIRE HYDRANT	TH THRESHOLD LEVEL
FL FLOOR LEVEL	TL TELEPHONE LEVEL
FS FLOOR SIGN	TP TELEGRAPH POLE
GA GATE	TRC TRAIL RECORDS
GC GATE VALVE	ULC UNABLE TO LOCATE
GD GATE VALVE	ULS UNABLE TO LOCATE
IC INFLECTOR CHAMBER	ULV UNABLE TO LOCATE
IN INVERT LEVEL	UT UNABLE TO TRACE
JB JUNCTION BOX	UV UNABLE TO TRACE
KB KERB LEVEL	VA VALVE
LB LETTER BOX	VB VALVE
LP LAMP POLE	WC WATER COCK
MR MANHOLE	WD WASH OUT VALVE
NT NOT LOCATED	WP WATER PIPE

b	BEAM HEIGHT
c	CELL HEIGHT
ch	CEILING HEIGHT
cl	CLIFF
cm	CORNER MARK
fm	FALSE CORNER HEIGHT
fl	FLOOR LEVEL
h	HEAD
hl	HEAD OVERHEAD LEVEL
h	HEAD HEIGHT
p	PUBLIC HEIGHT
r	RIDGE ROOF HEIGHT
sp	SPRING/HOLE
st	STAIRS
su	STAIRS UP

FENCE LINETYPES

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CT SURVEYS IS A MEMBER OF:

SURVEY NOTES:
The survey was carried out using GPS and has been drawn at a scale factor of 1:1000.
Data in the table below are not to scale and should only be used for reference purposes as shown on the drawing.

SURVEY REVISIONS

PREVIOUS DRAWING NO.	-
REVISION NO.	-
ORIGINAL SURVEY DATE	-

REVISION NOTES:

LAYOUT VIEWS

Cushman & Wakefield

PROJECT TITLE:
Land off Ryebank Rd/Langford Rd, Manchester, M21 9WW

DRAWING NUMBER:
4692AB/2

SCALE: 1:200 @ A0	SURVEY DATE: November 2019
LEVEL DATUM: OSGB by GPS	CREATED BY: OSGB fixed at ST06
BY: Niall at ST06 surveyed by GPS	DATE: 28.10.19
	PAGE: 2 of 5



Station	Easting	Northing	Notes
AP01	390939.492	394652.329	27.558
ST01	391074.450	394651.316	28.570
ST02	391000.324	394620.449	28.570
ST03	390970.415	394620.459	28.510
ST04	390940.306	394457.088	28.432
ST05	390951.168	394496.103	27.902
ST06	391004.552	394485.330	28.167
ST07	391016.219	394468.257	28.484
ST08	391057.507	394438.593	28.583
ST09	391016.214	394542.142	27.652
ST10	391140.918	394601.013	27.294
ST11	391191.144	394621.238	27.311
ST20	391138.303	394740.452	26.927
ST21	391138.312	394740.178	27.209

ABBREVIATIONS

AK	ASSURED ROUTE	OA	OUTSIDE SURVEY AREA
AV	ARM VALVE	P	POST POLE
B	BOLLARD	RL	ROOF LEVEL
BF	BENT PIPE	RF	ROOF FLOOR
BT	BEST FIT TELECOM CHAMBER	RS	RIDGE SIGN
CA	CABLE	RP	RAN/WATER PIPE
CAU	CABLE TV	S	STOP
CB	CABLE BOX	SC	STOP COOK
CL	COVER LEVEL	ST	STOP SIGN
CM	CABLE MESS	SV	STOP VALVE
CP	DOWN PIPE	SW	STOP VALVE PIPE
EL	LEVEL	SWP	STOP VALVE PIPE
EP	ELECTRICITY POLE	TC	TRAFFIC CONTROL
ER	EARTH ROD	TBR	TRAFFIC SIGN RECORD
FF	FIRE HYDRANT	TH	THRESHOLD LEVEL
FL	FLOOR LEVEL	TL	TRAFFIC LIGHT
FLC	FLOOR LEVEL	TP	TELEGRAPH POLE
GA	GAS VALVE	TR	TRAILER TO GAN ACCESS
GC	GAS VALVE	UL	UNABLE TO LOCATE
IC	INSPECTOR CHAMBER	ULS	UNABLE TO SURVEY
IL	INVERT LEVEL	UR	UNABLE TO TRACE
J	JUNCTION BOX	VS	VALVE LEVEL
KB	KEY HOLE	VP	VENT PIPE
LB	LETTER BOX	W	WATER MAIN
LP	LETTER BOX	WD	WASH OUT VALVE
MR	MANHOLE	WIP	WATER PIPE
NT	NOT LOCKED		
NT	NO TRACE		

FENCE LINETYPES

1	BEAM HEIGHT
2	CELL HEIGHT
3	CEILING HEIGHT
4	FALSE CEILING HEIGHT
5	FALSE CEILING HEIGHT
6	FLOOR LEVEL
7	BEAM OVERHEAD LEVEL
8	BEAM OVERHEAD LEVEL
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100	BEAM OVERHEAD LEVEL

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SURVEY NOTES:
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Check the scale factor for any setting out or design works should strictly use the control coordinates as shown on the drawing.

SURVEY REVISIONS

PREVIOUS DRAWING NO.	-
REVISION NO.	-
ORIGINAL SURVEY DATE	-

REVISION NOTES:

LAYOUT VIEWS

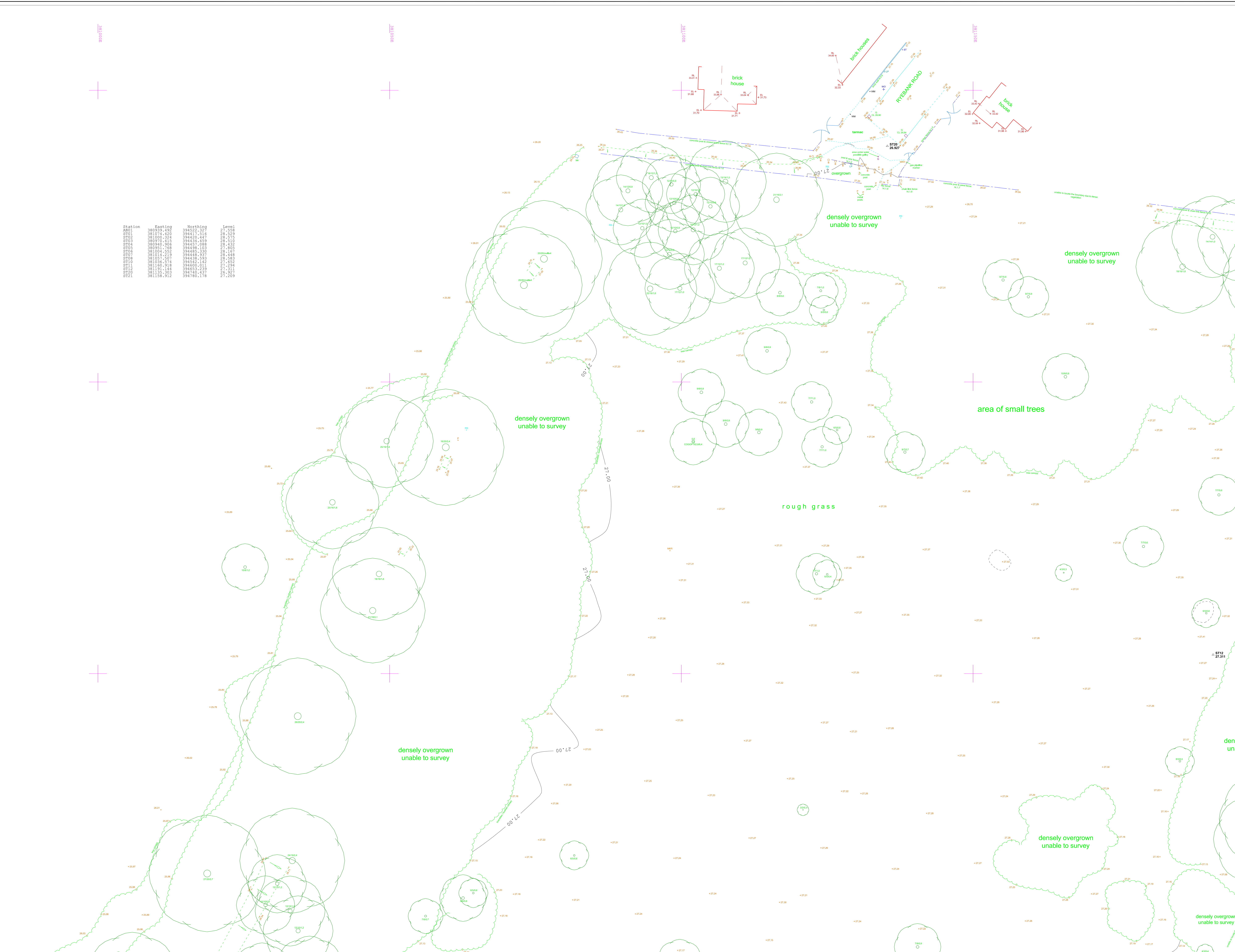
Cushman & Wakefield

PROJECT TITLE:
Land off Ryebank Rd/Langford Rd, Manchester, M21 9WW

DRAWING NUMBER:
4692AB/5

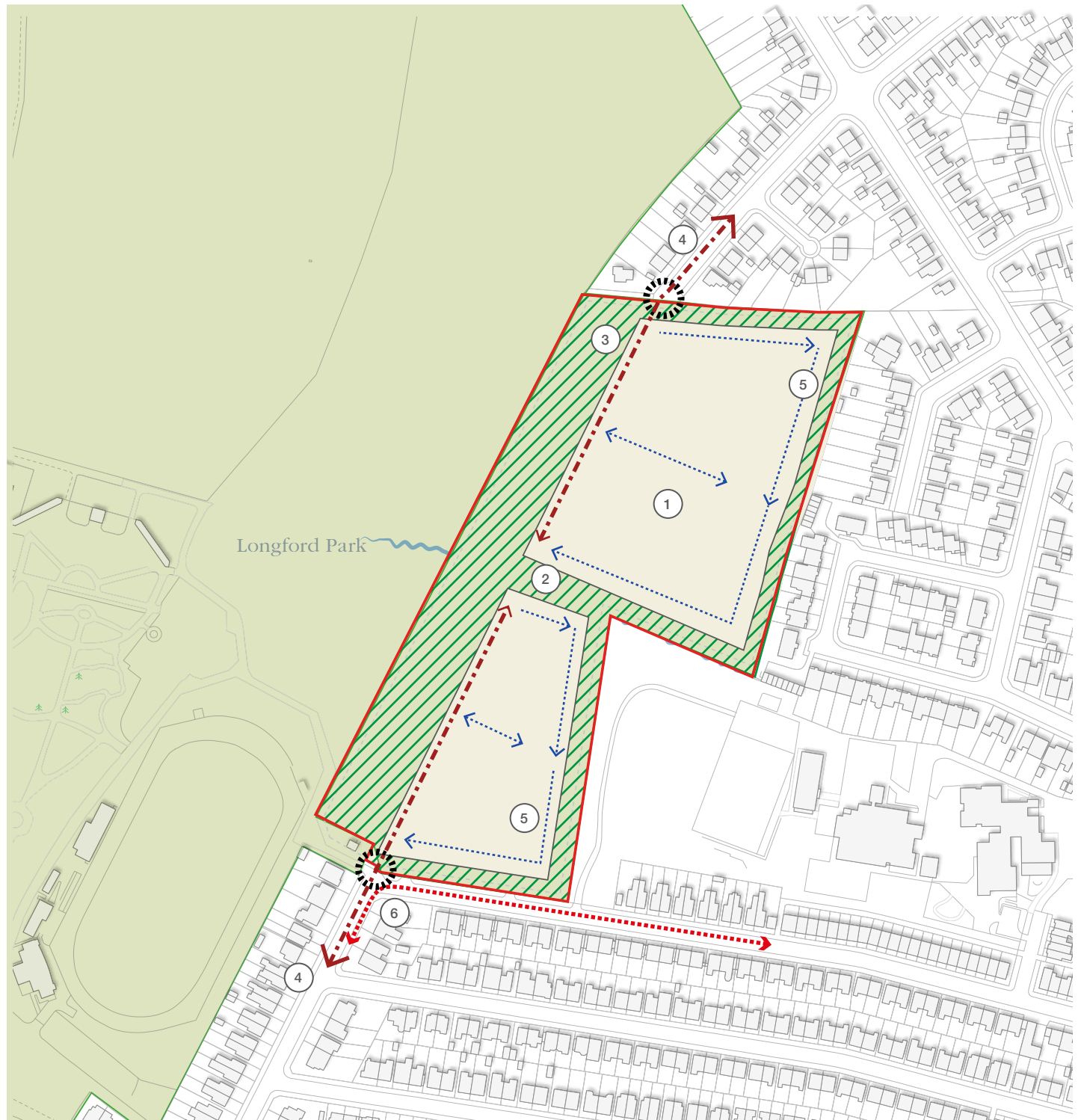
SCALE: 1:200 @ A0	SURVEY DATE: November 2019
LEVEL DATUM: OSGB by GPS	ORIENTATION: OSGB fixed at ST06
BENCH MARK USED: N44 at ST06 surveyed by GPS	MAP SCALE: 28.167m
	SHEET NUMBER: 5 of 5

Station	Easting	Northing	Level
ST01	380939.192	394927.127	27.308
ST02	381074.420	394417.516	28.529
ST03	381000.224	394620.417	28.575
ST04	380970.415	394434.459	28.510
ST05	380940.896	394485.188	28.412
ST06	380901.768	394496.103	27.805
ST07	381044.219	394485.130	28.147
ST08	381014.219	394449.937	28.448
ST09	381054.219	394485.130	28.583
ST10	381014.219	394485.130	27.853
ST11	381100.318	394600.211	27.274
ST12	381131.144	394651.239	27.111
ST13	381135.303	394740.437	26.927
ST14	381158.912	394760.178	27.209



APPENDIX 2: Development Framework

3.2 Summary of Opportunities

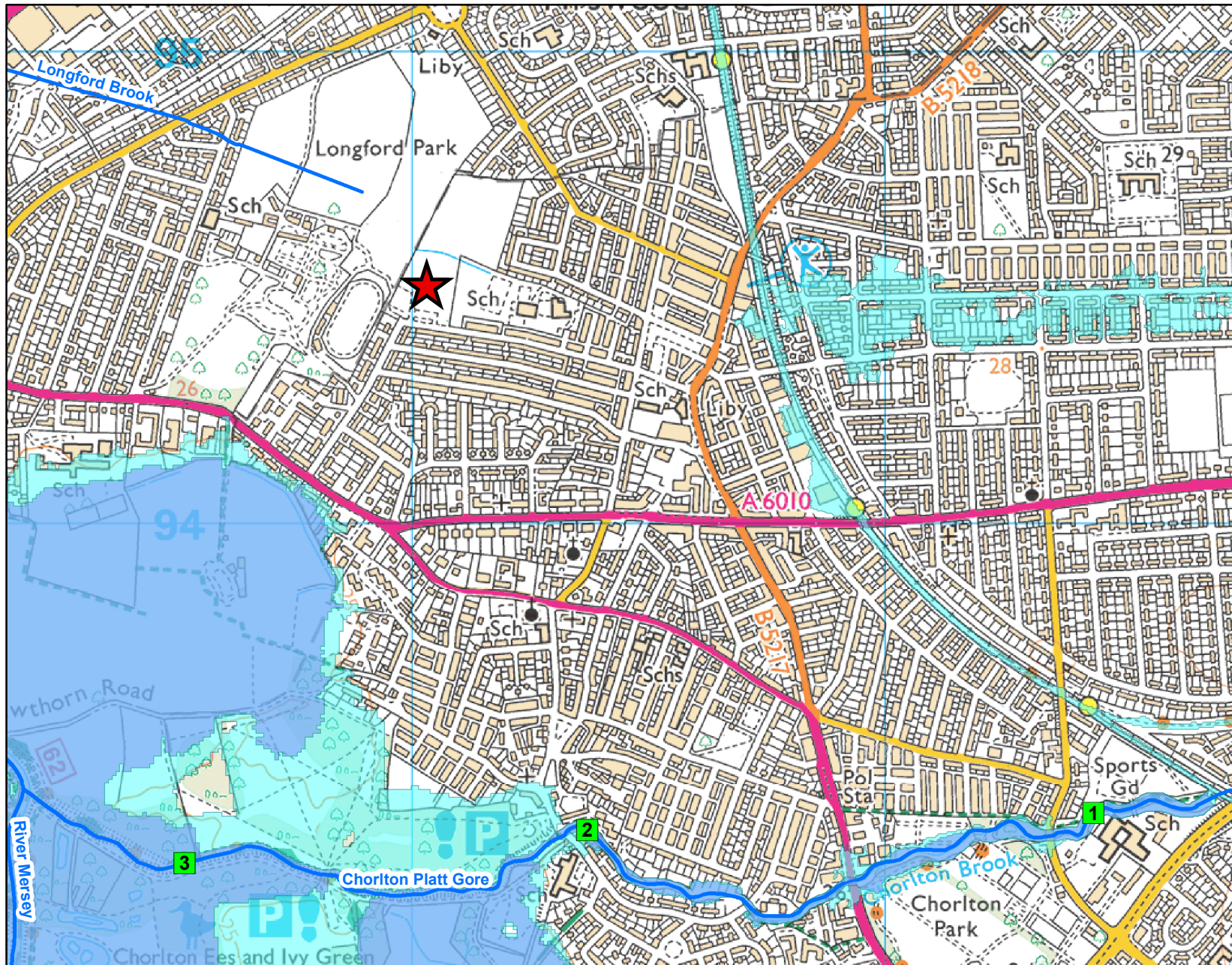


- Key**
- Site Boundary
 - 1. Improve Quality of Existing Open Space within any new development
 - 2. Increase Biodiversity with Use of SUDS
 - 3. Improve Pedestrian
 - 4. Park Access, Permeability and Safety Enhanced Pedestrian and Cycle Connectivity for the Benefit of Existing Neighbourhoods. Potential to introduce vehicle access from the North.
 - 5. Increased Safety around Development
 - 6. Potential Traffic Management

A summary of the site's opportunities are depicted adjacent.

APPENDIX 3: EA Correspondence






Detailed Flood Map centred on Ryebank Road, Chorlton, M21 9SN. Created on 31/10/2019 [GMMC147886AB]



1:12,001



Legend

-  Site Location
-  Model measurements
-  Main River
-  Flood Zone 3
-  Flood Zone 2

Map Reference	Model Node Reference	Easting	Northing	Data	Undefended								Defended							
					10 % AEP (1 in 10 year)	4 % AEP (1 in 25 year)	1.33 % AEP (1 in 75 year)	1 % AEP (1 in 100 year)	1 % AEP (1 in 100 year) + 30yr Climate Change	1 % AEP (1 in 100 year) + 35yr Climate Change	1 % AEP (1 in 100 year) + 70yr Climate Change	0.1 % AEP (1 in 1000 year)	10 % AEP (1 in 10 year)	4 % AEP (1 in 25 year)	1.33 % AEP (1 in 75 year)	1 % AEP (1 in 100 year)	1 % AEP (1 in 100 year) + 30yr Climate Change	1 % AEP (1 in 100 year) + 35yr Climate Change	1 % AEP (1 in 100 year) + 70yr Climate Change	0.1 % AEP (1 in 1000 year)
1	ea013_Model_CHOP01_00034	382443	393388	Modelled Water Level (m aodN)	26.03	26.39	26.78	26.86	27.29	27.35	27.46	27.48	26.03	26.39	26.79	26.87	27.33	27.37	27.46	27.49
				Modelled Flow (cumecs)	10.79	13.53	16.12	16.67	19.52	19.99	21.02	21.18	10.79	13.54	16.14	16.68	19.83	20.19	21.12	21.24
2	ea013_Model_CHOP01_00054	381376	393354	Modelled Water Level (m aodN)	24.44	24.51	24.58	24.59	24.70	24.71	24.75	24.78	24.44	24.51	24.58	24.59	24.70	24.72	24.75	24.78
				Modelled Flow (cumecs)	10.87	13.30	15.63	16.06	18.56	18.92	20.15	20.79	10.87	13.31	15.65	16.07	18.80	19.09	20.23	20.99
3	ea013_Model_CHOP01_00062	380518	393278	Modelled Water Level (m aodN)	24.34	24.35	24.36	24.36	24.39	24.39	24.40	24.38	24.34	24.35	24.36	24.36	24.39	24.39	24.40	24.38
				Modelled Flow (cumecs)	8.99	10.79	12.55	12.85	11.30	11.54	12.39	16.63	8.99	10.80	12.56	12.86	11.44	11.64	12.43	16.79

Model data taken from Chorlton Platt Gore 2012

AEP - Annual Exceedence Probability

m aodN - metres above ordnance datum Newlyn

cumecs - cubic metres per second

Notes: *Climate Change Scenario - 30%, 35% and 70% increases in flow calculated for the 2080's (2070 - 2115). Please see <https://www.gov.uk/guidance/flood-risk-assessments-climate-change-allowances> for more information regarding the new climate change guidance. The location of the site and the type (vulnerability) of development determine the climate change allowances to consider in any flood risk assessment.

Reservoir Flood Map




The area within the red circle could be at risk of flooding from the following reservoirs:

Reservoir Name	Reservoir Owner	Location	Local Authority	Environment Agency Office
Gorton Upper	United Utilities Water plc	389893, 396254	Manchester	Environment Agency - Greater Manchester, Merseysid
Audenshaw No. 3	United Utilities Water plc	390934, 396835	Tameside	Environment Agency - Greater Manchester, Merseysid
Audenshaw No.2	United Utilities Water plc	391643, 396093	Tameside	Environment Agency - Greater Manchester, Merseysid
Audenshaw No.1	United Utilities Water plc	391852, 396483	Tameside	Environment Agency - Greater Manchester, Merseysid

Note - this map provides a general indication of the largest area that might be flooded if a reservoir were to fail and release the water it holds. It is taken from a national assessment and displays a worst case scenario. The map is only intended as a guide and is not a prediction of what will happen.

Important

- This map has been produced for emergency planning purposes and displays a worst case scenario.
- It is not suitable for use at an individual property scale due to the method used.
- This map does not give any information on the likelihood of reservoir flooding or on the depth or speed of floodwaters. It also does not include any smaller reservoirs (which hold less than 25,000 cubic metres of water) or reservoirs commissioned or registered after Spring 2009.
- The information should not be interpreted as stating that the location you are interested in will or won't actually flood, but simply that it is in or not in an area that could be affected by reservoir flooding as shown on the maps.

 Maximum extent of flood

1:10,000

Reservoir Flood Map

This text must be read with the extract from the Reservoir Flood Map which we have sent to you

How to use the maps

Reservoir flood maps are available to help you find out if you could be affected by reservoir flooding. Even though reservoir flooding is very unlikely it may be helpful to you to find out if you live or work in an area that could be affected. If you do, you might want to think about what you would do if an emergency did happen.

For more information on what to do if you live or work near a reservoir, including some frequently asked questions, visit our website at <http://www.environment-agency.gov.uk/flood>.

The maps have been prepared for emergency planning purposes and for this reason they reflect a credible worst case scenario – this means that if a reservoir failure did occur it would most likely be far less severe than the scenario shown in the maps. We've mapped a credible worst case scenario so that emergency planners have all the information they might need to increase public safety.

Reservoir safety

Reservoirs in the UK have an extremely good safety record with no failures resulting in the loss of life since 1925. Reservoirs are more carefully maintained now. This means reservoir flooding is very unlikely to happen.

The Environment Agency is the enforcement authority for the Reservoirs Act 1975 in England. All large reservoirs that we think could endanger human life must be inspected and supervised by reservoir engineers. We ensure that reservoirs are regularly inspected and essential safety work is carried out.

For more information on reservoir safety visit our website at:

<https://www.gov.uk/guidance/reservoirs-owner-and-operator-requirements>.

Emergency planning

Lead Local flood authorities are responsible for coordinating emergency plans for reservoir flooding and ensuring communities are well prepared. Lead Local flood authorities work with other members of the Local Resilience Forum (LRF) to develop generic and site-specific emergency plans, depending on local circumstances and priorities.

If you want to find out about local emergency plans you should contact the responsible lead local flood authority as identified on the map.

APPENDIX 4: Manchester City Council Correspondence

Lucy Reeves

From: [REDACTED] on behalf of
floodriskmanagement@manchester.gov.uk
Sent: 30 October 2019 12:13
To: Lucy Reeves
Cc: floodriskmanagement@manchester.gov.uk
Subject: Re: Request for Flood Risk Information (Ryebank Road, Chorlton)
Attachments: We found suspicious links; Indicative Site Boundary.png

Good afternoon

We do not have record of any flooding on the site or in immediate vicinity of the site. From the OS maps it is visible that there is a drain that crosses the site - its connectivity and function needs to be investigated as part of the flood risk assessment. Information on sewer flooding could be obtained from United Utilities. Information on surface water and ground water flooding and mapping of their extent is available from the Environment Agency website.

We are attaching guidance for developers on requirements related to surface water management that you might find useful if you are also developing a drainage strategy for the site.

Regards

Flood Risk Management
Highways Service, Growth and Neighbourhoods Directorate
Manchester City Council
Tel: 0161 219 6295
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Lead Local Flood Authority - Manchester City Council

[SuDS Requirements for New Developments Update 2019](#)

The Government has strengthened planning policy on the provision of sustainable drainage systems (SuDS) for 'major' planning applications which is being introduced from 6 April 2015 (Paragraph 103 of National Planning Policy Framework and Ministerial Statement on SuDS).

Changes were made to the Town and Country Planning Policy and Guidance to give Local Authority Planning Departments the responsibility for ensuring that new developments are drained in a sustainable way, through the planning process, in consultation with the Lead Local Flood Authorities. As per the guidance issued by the Department of Communities and Local Government (DCLG), all 'major' planning applications being determined from 6 April 2015, must consider sustainable drainage systems.

Decisions about the suitability of sustainable drainage provision are made by the Local Planning Authority. However, under the new consultation arrangements Manchester City Council, in its role as Lead Local Flood Authority, is a statutory consultee for all major applications with regards to sustainable drainage. All 'major' planning applications submitted from 15 April 2015 are required to include a Surface Water Drainage Statement.

Developers submitting the planning application to Manchester City Council are advised to:

- Assess the suitability of sustainable drainage systems in accordance with: paragraphs 051, 079 and 080 of the revised NPPF Planning Practice Guidance (PPG) for Flood Risk and Coastal Change.
- Design sustainable drainage systems in line with national Non-Statutory Technical Standards for SuDS: <https://www.gov.uk/government/publications/sustainable-drainage-systems-non-statutory-technical-standards> and local policies DM1, EN08, EN14 and EN17 of the Core Strategy.
- Implement strategy that supports surface water runoff control as near to its source as possible through a sustainable drainage approach to surface water management, preferably through green types of SuDS wherever practicable.
- Maximise use of Green Infrastructure to manage surface water runoff (sustainable drainage systems – SuDS) as feasibility of use of green types of SuDS will be evaluated by LLFA during planning application stage. Even on sites where infiltration is not possible due to the unfavourable soil conditions, utilisation of attenuating types of SuDS should be maximised as part of green infrastructure in order to bring surface water runoff peak and volume reduction but also provide additional environmental and social benefits in line with Policies DM1, EN08, EN14 and EN17 of the Core Strategy. Developers are encouraged to look into CIRIA SuDS Manual 2015 for details of these systems to manage surface water runoff.
- Assess surface water attenuation requirements that offer a reduction in surface water runoff rate in line with the 'Manchester City, Salford City and Trafford Councils Level 2 Hybrid SFRA, User Guide, Final, dated May 2010'. (Section 3.4.1 Critical Drainage Areas, page 31). Please note that all new connections to the watercourses shall comply with reduction of flows to Greenfield runoff rates.

http://www.manchester.gov.uk/downloads/download/3871/strategic_flood_risk_assessment-manchester_salford_trafford

- Submit hydraulic calculations and drawings to support the design along with proposed standards of operation and maintenance in accordance with paragraph 081 of NPF (PPG).
- It will be essential that the type of sustainable drainage system for a site, along with details of its extent/position, is identified at the design stage of the whole scheme. This information will be required for both outline and full applications so it is clearly demonstrated that the SuDS can be accommodated within the development that is proposed. It will no longer be acceptable to leave the design of SuDS to a later stage to be dealt with by planning conditions.

Applicants are strongly advised to discuss their proposals with relevant planning officers at the pre-application stage to ensure that an acceptable SuDS scheme is submitted.

Minimum requirements for approval

In order to avoid objection, the following three elements of evidence are required:

- Surface Water Management Statement for the site is submitted that is in line with requirement of NPPF PPG Paragraph 079:
 - “New development should only be considered appropriate in areas at risk of flooding if priority has been given to the use of green types of sustainable drainage systems as a primary mean of surface water management that could be accompanied by traditional attenuation systems if volumes required. Additionally, and more widely, when considering major development, as defined in the Town and Country Planning (Development Management Procedure) (England) Order 2015, sustainable drainage systems should be provided unless demonstrated to be inappropriate”.
 - The strategy should be accompanied by at least an outline layout of the proposed drainage systems with space allocation for the proposed attenuation in line with relevant flow reduction requirements.
- Evidence that drainage hierarchy has been applied in line with NPPF PPG Paragraph 080. The aim should be to discharge surface run off as high up the following hierarchy of drainage options as reasonably practicable:
 - into the ground (infiltration);
 - to a surface water body;
 - to a surface water sewer, highway drain, or another drainage system;
 - to a combined sewer.
- Evidence that one viable solution for draining the site is secured and agreed in principle with relevant authority:
 - Results of ground investigation carried out under Building Research Establishment Digest 365. Site investigations should be undertaken in locations and at proposed depths of the proposed infiltration devices. Proposal of the attenuation that is achieving half emptying time within 24 hours. If no ground

investigations are possible or infiltration is not feasible on site, evidence of alternative surface water disposal routes (as follows) is required.

- Where surface water is connected to Main River, any works within or adjacent to the river that would affect it would require consent from Environment Agency. An email of acceptance of proposed flows and/or new connection will suffice.
- Where surface water is connected to the public sewer, agreement in principle from United Utilities is required that there is adequate spare capacity in the existing system taking future development requirements into account. An email of acceptance of proposed flows and/or new connection will suffice.

Where surface water is connected to the ordinary watercourse, agreement in principle from Manchester City Council as Lead Local Flood Authority is required. Please note that all new connections to the watercourses shall comply with reduction of flows to Greenfield runoff rates. An email of acceptance of proposed flows and/or new connection will suffice.

- Where surface water is connected to ordinary watercourse, any works within or adjacent to the watercourse that would affect it would require consent from Manchester City Council as Lead Local Flood Authority. Consent forms can be obtained on the website at:

http://www.manchester.gov.uk/downloads/download/5567/land_drainage_consent_and_guidance.com

Avoiding or discharging drainage condition

- Evidence of consideration of green SuDS solution wherever practicable and viable in line with NPPF PPG Paragraph 051. SuDS should be designed to:
 - reduce the causes and impacts of flooding;
 - remove pollutants from urban run-off at source;
 - combine water management with green space with benefits for amenity, recreation and wildlife wherever practicable.
- For Greenfield site developments, details of surface water attenuation that offers a reduction in surface water runoff rate in line with the Manchester Trafford and Salford Strategic Flood Risk Assessment, i.e. to the Greenfield runoff rates;
- For Brownfield site developments, for all sites located within Conurbation Core Critical Drainage Areas, details of surface water attenuation that offers a reduction in surface water runoff rate in line with the Manchester Trafford and Salford Strategic Flood Risk Assessment, i.e. at least a 50% reduction in runoff rate compared to the existing rates with an aim to reduce to Greenfield runoff rates wherever practicable;
- Where surface water runoff is to be disposed through infiltration, details of surface water attenuation that is adequate for infiltration capacity of the underlying soil is required. This capacity has to be assessed through site investigation carried out in line with Building Research Establishment Digest 365. Site investigations should be undertaken in locations and at proposed depths of the proposed infiltration devices. Half emptying time of proposed attenuation within 24 hours should be achieved.

- Evidence that the drainage system has been designed (unless an area is designated to hold and/or convey water as part of the design) so that flooding does not occur during a 1 in 100 year rainfall event in any part of a building; The NPPF suggests that ‘for events with a return-period in excess of 30 years, surface flooding of open spaces such as landscaped areas or car parks is acceptable for short periods, but the layout and landscaping of the site should aim to route water away from any vulnerable property, and avoid creating hazards to access and egress routes. No flooding of property should occur as a result of a one in 100 year storm event (including an appropriate allowance for climate change)’.
- Assessment of overland flow routes for extreme events that is diverted away from buildings providing long and cross sections for the proposed drainage system and finished floor levels;
- Evidence that runoff volume in the 1 in 100 year, 6 hours rainfall shall be constrained to a value as close as is reasonably practical to the Greenfield runoff volume for the same event, but never to exceed the runoff volume from the development site prior to redevelopment;
- Hydraulic calculation of the proposed drainage system;
- Design construction drawings of proposed SuDS elements and flow controls;
- For sites where proposed development would cause unusual pollution risk to surface water (large car park areas (>50 parking spaces) or industrial estates), evidence of pollution control measures (preferably through SuDS) is required.
- Where an application is part of a larger site which already has planning permission it is essential that the new proposal does not compromise the drainage scheme already approved.

SuDS Maintenance

Maintenance of SuDS is essential for its proper operation and a clear management and maintenance plan for the lifetime of the development is required as part of the planning application.

In considering a development that includes a sustainable drainage system, Manchester City Council as local planning authority will want to be satisfied that the proposed minimum standards of operation are appropriate and that there are clear arrangements in place for ongoing maintenance. Information sought by Manchester City Council would be no more than necessary, having regard to the nature and scale of the development concerned in line with NPPF Paragraph 081.

In some instances where no clear adoption of the drainage system is proposed, a drainage maintenance condition could be attached to the planning applications. In order to discharge this condition, the following evidence should be provided following *construction*:

- Verification report providing photographic evidence of construction as per design drawings;
- As built construction drawings if different from design construction drawings;

- Management and maintenance plan for the lifetime of the development which shall include the arrangements for adoption by any public body or statutory undertaker, or any other arrangements to secure the operation of the sustainable drainage scheme throughout its lifetime.

Applicants are strongly advised to discuss their proposals with relevant planning officers at the pre-application stage to ensure that an acceptable SuDS scheme is submitted.

APPENDIX 5: Sewer Records

BWB Consulting Ltd

**5th Floor, Waterfront House
Station Street,
Nottingham,
NG23DQ**

FAO:

How to contact us:

**United Utilities Water Limited
Property Searches
Haweswater House
Lingley Mere Business Park
Great Sankey
Warrington
WA5 3LP**

Telephone: 0370 7510101

E-mail: propertysearches@uuplc.co.uk

Your Ref: MCW2136_POR028705

Our Ref: UUPS-ORD-131189

Date: 28/10/2019

Dear Sirs

Location: Ryebank Fields

I acknowledge with thanks your request dated 25/10/2019 for information on the location of our services.

Please find enclosed plans showing the approximate position of United Utilities' apparatus known to be in the vicinity of this site.

The enclosed plans are being provided to you subject to the United Utilities terms and conditions for both the wastewater and water distribution plans which are shown attached.

If you are planning works anywhere in the North West, please read United Utilities' access statement before you start work to check how it will affect our network. <http://www.unitedutilities.com/work-near-asset.aspx>.

I trust the above meets with your requirements and look forward to hearing from you should you need anything further.

If you have any queries regarding this matter please [contact us](#).

Yours Faithfully,



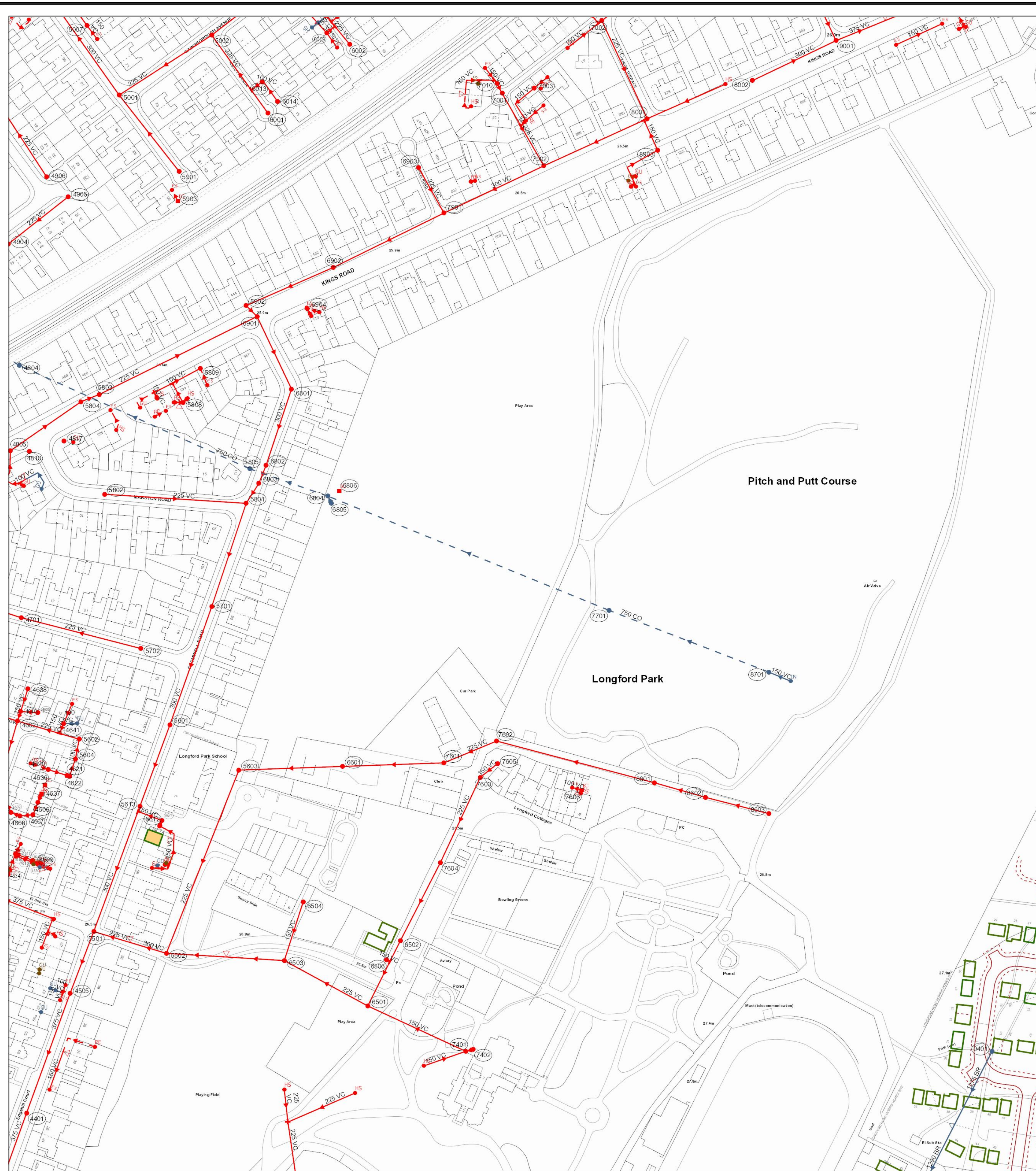
Karen McCormack
Property Searches Manager

TERMS AND CONDITIONS - WASTEWATER AND WATER DISTRIBUTION PLANS

These provisions apply to the public sewerage, water distribution and telemetry systems (including sewers which are the subject of an agreement under Section 104 of the Water Industry Act 1991 and mains installed in accordance with the agreement for the self construction of water mains) (UUWL apparatus) of United Utilities Water Limited "(UUWL)".

TERMS AND CONDITIONS:

- This Map and any information supplied with it is issued subject to the provisions contained below, to the exclusion of all others and no party relies upon any representation, warranty, collateral contract or other assurance of any person (whether party to this agreement or not) that is not set out in this agreement or the documents referred to in it.
- This Map and any information supplied with it is provided for general guidance only and no representation, undertaking or warranty as to its accuracy, completeness or being up to date is given or implied.
- In particular, the position and depth of any UUWL apparatus shown on the Map are approximate only. UUWL strongly recommends that a comprehensive survey is undertaken in addition to reviewing this Map to determine and ensure the precise location of any UUWL apparatus. The exact location, positions and depths should be obtained by excavation trial holes.
- The location and position of private drains, private sewers and service pipes to properties are not normally shown on this Map but their presence must be anticipated and accounted for and you are strongly advised to carry out your own further enquiries and investigations in order to locate the same.
- The position and depth of UUWL apparatus is subject to change and therefore this Map is issued subject to any removal or change in location of the same. The onus is entirely upon you to confirm whether any changes to the Map have been made subsequent to issue and prior to any works being carried out.
- This Map and any information shown on it or provided with it must not be relied upon in the event of any development, construction or other works (including but not limited to any excavations) in the vicinity of UUWL apparatus or for the purpose of determining the suitability of a point of connection to the sewerage or other distribution systems.
- No person or legal entity, including any company shall be relieved from any liability howsoever and whensoever arising for any damage caused to UUWL apparatus by reason of the actual position and/or depths of UUWL apparatus being different from those shown on the Map and any information supplied with it.
- If any provision contained herein is or becomes legally invalid or unenforceable, it will be taken to be severed from the remaining provisions which shall be unaffected and continue in full force and affect.
- This agreement shall be governed by English law and all parties submit to the exclusive jurisdiction of the English courts, save that nothing will prevent UUWL from bringing proceedings in any other competent jurisdiction, whether concurrently or otherwise.



Refo	Cover	Func	Invert	Size x	Size y	Shape	Mat	Length	Grad
5702	26.46	CO	24.89	225			VC	68.5424	1 in 158
5801	25.61	CO	23.47	300			VC	60.08327	1 in 462
5804				100			VC	11.15891	
6013				100			VC	7.442999	
8001	26.2	CO	24.87	225			VC	89.88889	
4636				150			VC	13.02442	
6504				150			VC	33.7778	
4540				150			VC	5.50341	
5811				150			VC	14.06757	
7601	26.55	CO	24.56	225			VC	57.1664	1 in 672
4401	26.56	CO	22.05	375			VC	76.55064	1 in 273
4401	26.56	CO	22.05	375			VC	76.55064	1 in 273
4805	25.83	CO	24.39	225			VC	47.39897	1 in 190
6001	26.2	CO	24.9	225			VC	53.02443	1 in 133
4701	26.39	CO	24.46	225			VC	48.02448	
7601	26.39	CO	24.46	225			VC	78.74643	1 in 54
7602	26.53	CO	25.33	225			VC	67.97794	1 in 971
6002	26.53	CO	25.33	225			VC	50.44799	1 in 126
6506				150			VC	2.13211	
6506				150			VC	2.13211	
5002	26.1	CO	24.5	225			VC	60.74537	1 in 148
8602	26.18	CO	24.87	225			VC	29.20444	
4628				100			PVC	3.765324	
7602	25.89	CO	24.63	225			VC	31.56471	1 in 448
7601	26.55	CO	24.56	225			VC	55.90437	1 in 569
8007				150			VC	18.22288	
8701				750			CO	84.20961	
8602	26.86	CO	23.97	300			VC	52.39275	1 in 476
4602	26.17	CO	24.42	225			VC	25	1 in 25
4505	26.51	CO	22.26	375			VC	10.83014	
4505	26.51	CO	22.26	375			VC	70.2282	1 in 334
5802	25.85	CO	23.86	300			VC	8.485291	1 in 85
8502	26.91	CO	25.05	225			VC	40.24929	1 in 180
8502	26.91	CO	25.05	225			VC	40.24929	1 in 149
4607				150			VC	7.31554	
8603	26.14	CO	25.35	225			VC	36.13862	1 in 75
5001	25.87	CO	24.95	300			VC	78.77971	
5804	25.77	CO	24.14	225			VC	10.77033	1 in 215
4905	25.65	CO	24.62	225			VC	42.61169	
6014				100			VC	13.98761	
5802	25.79	CO	24.05	225			VC	78.3601	1 in 147
8804	25.13	SW	23.81	750			CO	65.81199	1 in 651
7002	26.65	CO	24.95	225			VC	59.5093	1 in 165
5612				150			VC	3.25226	
5805	25.55	SW	23.74	750			CO	139.2049	
7001	26.44	SW	24.79	750			CO	68.14109	1 in 76
7701				150			VC	167.3141	
8009				1000			CO	8.806913	
4604				100			VC	36.39923	
6503	26.87	CO	24.37	225			VC	32.02444	
6503	26.87	CO	24.37	225			VC	32.02444	
7902	26.43	CO	24.13	300			VC	60.83585	
7605				150			VC	12.07717	
4622				100			VC	9.936145	
7010				150			VC	6.036557	
4617				100			PVC	1.631608	
6801	25.64	CO	23.63	300			VC	44.27189	
4608				150			VC	5.174318	
5502	26.82	CO	23.9	300			VC	20.80601	1 in 29
5002	26.82	CO	23.9	300			VC	20.80601	1 in 29
5803	25.74	CO	24.09	225			VC	97.04838	1 in 313
4616				100			PVC	6.156486	
6805	25.78	SW	24.44	225			VC	4.472136	1 in 7
4639				100			VC	9.56617	
8002	26.87	CO	25.13	300			VC	59.9902	1 in 59
8603	25.9	CO	24.43	225			VC	26.6531	1 in 92
4621				100			VC	1.532718	
4606				100			VC	7.202488	
7003				150			VC	24.46837	
7603	26.6	CO	25.55	225			VC	51.88412	1 in 124
8001	26.81	CO	24.99	300			VC	52.64862	1 in 142
6601	25.91	CO	23.76	300			VC	44.26318	738
5806				100			VC	12.02447	
5701	26.01	CO	23.34	300			VC	68.94926	1 in 406
6501	27.02	CO	24.78	225			VC	52.35456	1 in 128
6501	27.02	CO	24.78	225			VC	52.35456	1 in 128
4904				0			VC	45.48626	
8603	25.48	CO	23.36	300			VC	13.0244	1 in 145
7401	27.4	CO	26.02	150			VC	59.5093	1 in 48
7401	27.4	CO	26.02	150			VC	59.5093	1 in 48
7604	26.49	CO	25.23	225			VC	48.26314	1 in 268
8602	25.53	CO	24.30	300			VC	10.77033	
9001	27.04	CO	24.26	375			VC	26.15476	1 in 208
8603				150			VC	18.24982	
5601	26.37	CO	23.17	300			VC	73.62277	
0401	27.97	SW	22.54	1275			BR	48.30114	1 in 4830
0401	27.97	SW	22.54	1275			BR	48.30114	1 in 4830
0401	27.97	SW	22.54	1275			BR	48.30114	1 in 4830
0401	27.97	SW	22.54	1275			BR	48.30114	1 in 4830
4609				150			VC	7.226205	
5901	25.77	CO	24.67	225			VC	53.41348	1 in 92
7402	27.3	CO	26.08	150			VC	4.123106	1 in 69
7402	27.3	CO	26.08	150			VC	4.123106	1 in 69
4806	25.63	CO	24.38	225			VC	54.91912	
5603	26.91	CO	24.46	225			VC	108.6324	1 in 205
4641				150			VC	5.948379	
5602	26.22	CO	24.83	225			VC	36.44009	1 in 93
5501	26.45	CO	22.44	375			VC	36.40055	1 in 202
5501	26.45	CO	22.44	375			VC	36.40055	1 in 202

Refo	Cover	Func	Invert	Size x	Size y	Shape	Mat	Length	Grad
5801				225			VC	68.5424	1 in 158
5801				225			VC	60.08327	1 in 462
5804				100			VC	11.15891	
6013				100			VC	7.442999	
8001				225			VC	89.88889	
4636				150			VC	13.02442	
6504				150			VC	33.7778	
4540				150			VC	5.50341	
5811				150			VC	14.06757	
7601				225			VC	57.1664	1 in 672
4401				375			VC	76.55064	1 in 273
4401				375			VC	76.55064	1 in 273
4805				225			VC	47.39897	1 in 190
6001				225			VC	53.02443	1 in 133
4701				225			VC	48.02448	
7601				225			VC	78.74643	1 in 54
7602				225			VC	67.97794	1 in 971
6002				225			VC	50.44799	1 in 126
6506				150			VC	2.13211	
6506				150			VC	2.13211	
5002				225			VC	60.74537	1 in 148
8602				225			VC	29.20444	
4628				100			PVC	3.765324	
7602				225			VC	31.56471	1 in 448
7601				225			VC	55.90437	1 in 569
8007				150			VC	18.22288	
8701				750			CO	84.20961	
8602				300			VC	52.39275	1 in 476
4602				225			VC	25	1 in 25
4505				375			VC	10.83014	
4505				375			VC	70.2282	1 in 334
5802				300			VC	8.485291	1 in 85
8502				225			VC	40.24929	1 in 180
8502				225			VC	40.24929	1 in 149
4607				150			VC	7.31554	
8603				225			VC	36.13862	1 in 75
5001				300			VC	78.77971	
5804				225			VC	10.77033	1 in 215
4905				225			VC	42.61169	
6014				100			VC	13.98761	
5802				225			VC	78.3601	1 in 147
8804				750			CO	65.81199	1 in 651
7002				225			VC	59.5093	1 in 165
5612				150			VC	3.25226	
5805				750			CO	139.2049	
7001				750			CO	68.14109	1 in 76
7701				150			VC	167.3141	
8009				1000			CO	8.806913	
4604				100			VC	36.39923	
6503				225			VC	32.02444	
6503				225			VC	32.02444	
7902				300			VC	60.83585	
7605				150			VC	12.07717	
4622				100			VC	9.936145	
7010				150			VC	6.036557	
4617				100			PVC	1.631608	
6801				300			VC	44.27189	
4608				150			VC	5.174318	
5502				300			VC	20.80601	1 in 29
5002				300			VC	20.80601	1 in 29
5803				225			VC	97.04838	1 in 313
4616				100			PVC	6.156486	
6805				225			VC	4.472136	1 in 7
463									



Refno	Cover	Func	Invert	Size x	Size y	Shape	Matl	Length	Grad
8501	CO	0	24.37	225	150	VC	6.280562	1.44	
8502	CO	0	24.37	225	150	VC	30.07707		
8503	CO	0	24.37	225	150	VC	60.372655		
8504	CO	0	24.37	225	150	VC	72.092022		
8505	CO	0	24.37	225	150	VC	83.811389		
8506	CO	0	24.37	225	150	VC	95.530756		
8507	CO	0	24.37	225	150	VC	107.250122		
8508	CO	0	24.37	225	150	VC	118.969489		
8509	CO	0	24.37	225	150	VC	130.688855		
8510	CO	0	24.37	225	150	VC	142.408222		
8511	CO	0	24.37	225	150	VC	154.127588		
8512	CO	0	24.37	225	150	VC	165.846955		
8513	CO	0	24.37	225	150	VC	177.566321		
8514	CO	0	24.37	225	150	VC	189.285688		
8515	CO	0	24.37	225	150	VC	201.005054		
8516	CO	0	24.37	225	150	VC	212.724421		
8517	CO	0	24.37	225	150	VC	224.443787		
8518	CO	0	24.37	225	150	VC	236.163154		
8519	CO	0	24.37	225	150	VC	247.882520		
8520	CO	0	24.37	225	150	VC	259.601887		
8521	CO	0	24.37	225	150	VC	271.321253		
8522	CO	0	24.37	225	150	VC	283.040620		
8523	CO	0	24.37	225	150	VC	294.759986		
8524	CO	0	24.37	225	150	VC	306.479353		
8525	CO	0	24.37	225	150	VC	318.198719		
8526	CO	0	24.37	225	150	VC	329.918086		
8527	CO	0	24.37	225	150	VC	341.637452		
8528	CO	0	24.37	225	150	VC	353.356819		
8529	CO	0	24.37	225	150	VC	365.076185		
8530	CO	0	24.37	225	150	VC	376.795552		
8531	CO	0	24.37	225	150	VC	388.514918		
8532	CO	0	24.37	225	150	VC	400.234285		
8533	CO	0	24.37	225	150	VC	411.953651		
8534	CO	0	24.37	225	150	VC	423.673018		
8535	CO	0	24.37	225	150	VC	435.392384		
8536	CO	0	24.37	225	150	VC	447.111751		
8537	CO	0	24.37	225	150	VC	458.831117		
8538	CO	0	24.37	225	150	VC	470.550484		
8539	CO	0	24.37	225	150	VC	482.269850		
8540	CO	0	24.37	225	150	VC	493.989217		
8541	CO	0	24.37	225	150	VC	505.708583		
8542	CO	0	24.37	225	150	VC	517.427950		
8543	CO	0	24.37	225	150	VC	529.147316		
8544	CO	0	24.37	225	150	VC	540.866683		
8545	CO	0	24.37	225	150	VC	552.586049		
8546	CO	0	24.37	225	150	VC	564.305416		
8547	CO	0	24.37	225	150	VC	576.024782		
8548	CO	0	24.37	225	150	VC	587.744149		
8549	CO	0	24.37	225	150	VC	599.463515		
8550	CO	0	24.37	225	150	VC	611.182882		
8551	CO	0	24.37	225	150	VC	622.902248		
8552	CO	0	24.37	225	150	VC	634.621615		
8553	CO	0	24.37	225	150	VC	646.340981		
8554	CO	0	24.37	225	150	VC	658.060348		
8555	CO	0	24.37	225	150	VC	669.779714		
8556	CO	0	24.37	225	150	VC	681.499081		
8557	CO	0	24.37	225	150	VC	693.218447		
8558	CO	0	24.37	225	150	VC	704.937814		
8559	CO	0	24.37	225	150	VC	716.657180		
8560	CO	0	24.37	225	150	VC	728.376547		
8561	CO	0	24.37	225	150	VC	740.095913		
8562	CO	0	24.37	225	150	VC	751.815280		
8563	CO	0	24.37	225	150	VC	763.534646		
8564	CO	0	24.37	225	150	VC	775.254013		
8565	CO	0	24.37	225	150	VC	786.973379		
8566	CO	0	24.37	225	150	VC	798.692746		
8567	CO	0	24.37	225	150	VC	810.412112		
8568	CO	0	24.37	225	150	VC	822.131479		
8569	CO	0	24.37	225	150	VC	833.850845		
8570	CO	0	24.37	225	150	VC	845.570212		
8571	CO	0	24.37	225	150	VC	857.289578		
8572	CO	0	24.37	225	150	VC	869.008945		
8573	CO	0	24.37	225	150	VC	880.728311		
8574	CO	0	24.37	225	150	VC	892.447678		
8575	CO	0	24.37	225	150	VC	904.167044		
8576	CO	0	24.37	225	150	VC	915.886411		
8577	CO	0	24.37	225	150	VC	927.605777		
8578	CO	0	24.37	225	150	VC	939.325144		
8579	CO	0	24.37	225	150	VC	951.044510		
8580	CO	0	24.37	225	150	VC	962.763877		
8581	CO	0	24.37	225	150	VC	974.483243		
8582	CO	0	24.37	225	150	VC	986.202610		
8583	CO	0	24.37	225	150	VC	997.921976		
8584	CO	0	24.37	225	150	VC	1009.641343		
8585	CO	0	24.37	225	150	VC	1021.360709		
8586	CO	0	24.37	225	150	VC	1033.080076		
8587	CO	0	24.37	225	150	VC	1044.799442		
8588	CO	0	24.37	225	150	VC	1056.518809		
8589	CO	0	24.37	225	150	VC	1068.238175		
8590	CO	0	24.37	225	150	VC	1079.957542		
8591	CO	0	24.37	225	150	VC	1091.676908		
8592	CO	0	24.37	225	150	VC	1103.396275		
8593	CO	0	24.37	225	150	VC	1115.115641		
8594	CO	0	24.37	225	150	VC	1126.835008		
8595	CO	0	24.37	225	150	VC	1138.554374		
8596	CO	0	24.37	225	150	VC	1150.273741		
8597	CO	0	24.37	225	150	VC	1161.993107		
8598	CO	0	24.37	225	150	VC	1173.712474		
8599	CO	0	24.37	225	150	VC	1185.431840		
8600	CO	0	24.37	225	150	VC	1197.151207		
8601	CO	0	24.37	225	150	VC	1208.870573		
8602	CO	0	24.37	225	150	VC	1220.589940		
8603	CO	0	24.37	225	150	VC	1232.309306		
8604	CO	0	24.37	225	150	VC	1244.028673		
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8606	CO	0	24.37	225	150	VC	1267.467406		
8607	CO	0	24.37	225	150	VC	1279.186772		
8608	CO	0	24.37	225	150	VC	1290.906139		
8609	CO	0	24.37	225	150	VC	1302.625505		
8610	CO	0	24.37	225	150	VC	1314.344872		
8611	CO	0	24.37	225	150	VC	1326.064238		
8612	CO	0	24.37	225	150	VC	1337.783605		
8613	CO	0	24.37	225	150	VC	1349.502971		
8614	CO	0	24.37	225	150	VC	1361.222338		
8615	CO	0	24.37	225	150	VC	1372.941704		
8616	CO	0	24.37	225	150	VC	1384.661071		
8617	CO	0	24.37	225	150	VC	1396.380437		
8618	CO	0	24.37	225	150	VC	1408.100004		
8619	CO	0	24.37	225	150	VC	1419.819370		
8620	CO	0	24.37	225	150	VC	1431.538737		
8621	CO	0	24.37	225	150	VC	1443.258103		
8622	CO	0	24.37	225	150	VC	1454.977470		
8623	CO	0	24.37	225	150	VC	1466.696836		
8624	CO	0	24.37	225	150	VC	1478.416203		
8625	CO	0	24.37	225	150	VC	1490.135569		
8626	CO	0	24.37	225	150	VC	1501.854936		
8627	CO	0	24.37	225	150	VC	1513.574302		
8628	CO	0	24.37	225	150	VC	1525.293669		
8629	CO	0	24.37	225	150	VC	1537.013035		
8630	CO	0	24.37	225	150	VC	1548.732402		
8631	CO	0	24.37	225	150	VC	1560.451768		
8632	CO	0	24.37	225	150	VC	1572.171135		
8633	CO	0	24.37	225	150	VC	1583.890501		
8634	CO	0	24.37	225	150	VC	1595.609868		
8635	CO	0	24.37	225	150	VC	1607.329234		
8636	CO	0	24.37	225	150	VC	1619.048601		
8637	CO	0	24.37	225	150	VC	1630.767967		
8638	CO	0	24.37	225	150	VC	1642.487334		
8639	CO	0	24.37	225	150	VC	1654.206700		
8640	CO	0	24.37	225	150	VC	1665.926067		
8641	CO	0	24.37	225	150	VC	1677.645433		
8642	CO	0	24.37	225	150	VC	1689.364800		
8643	CO	0	24.37	225	150	VC	1701.084166		
8644	CO	0	24.37	225	150	VC	1712.803533		
8645	CO	0	24.37	225	150	VC	1724.522900		
8646	CO	0	24.37	225	150	VC	1736.242266		
8647	CO	0	24.37	225	150	VC	1747.961633		
8648	CO	0	24.37	225	150	VC	1759.680999		
8649	CO	0	24.37	22					

APPENDIX 6: Drain Alert Report



Cripplegate Lane, Hoghton, Preston. PR5 0RR
Tel: 01254 851500 Fax: 01254854004 service@drain-alert.co.uk

BWB Consulting Ltd
11 Portland Street
Manchester
M1 3HU

3rd March 2020

Dear Keith,

Reference: - JN29928, Ryebank Fields.

May we thank you for your valued custom. As requested, we have conducted a CCTV inspection at the above premises. We have emailed a link to you to access the Wincan VX video clips and documents via the cloud.

We trust that the report is to your satisfaction; however, should you have any queries then please do not hesitate to contact me.

Yours sincerely,

Mr S W Ormisher, B.A.(Hons.),
Technical Services Consultant

Service areas: Preston • Bolton • Wigan • Salford • Tameside • Rochdale • Cheshire • Fylde • Burnley
Company Reg. No. 029502950360 • Reg. No. 448 2116 57





Plan of the drainage system, not to scale
Enclosed

Conclusion

As requested, a CCTV survey and investigation of United Utilities surface water drainage lines was carried out. Upon arrival visible inspection found various manholes to be in light road, footpath and field areas, further investigation found silt debris to be minimal in manholes, therefore no jetting was carried out prior to the survey.

The survey was conducted upstream and downstream from manholes marked and identified on plan accordingly. Evidence from the survey found drainage lines to be of 1275mm brick construction throughout.

The general condition of lines surveyed was found to be reasonable and in expected working order throughout although one fault was seen which will require remedial work to be carried out to prevent problems occurring in the future.

Faults Found:

Section 1 UUMH1704-UUMH0401

Obstruction seen at 63.60mtrs causing reduced efficiency within the drain, restricting the survey.

Recommendation is to use the high-pressure tanker wagon to remove the debris allowing the CCTV to be carried out successfully.

We trust that the above is acceptable; however, should you require any further information, please do not hesitate to contact me.

Yours sincerely,

A handwritten signature in blue ink, appearing to read "S W Ormisher".

Mr S W Ormisher,
Technical Services Consultant



Disclaimer - Please note that any dimensions, levels and drainage layout drawings that are provided by Drain Alert, should be checked before being relied upon. All updated drawings are not to scale. It is the responsibility of the client to verify all information given with regards to the drainage prior to commencing any design or work site.



Project

Project Name: 29928 Ryebank Fields
Project Description: Converted project from v8 project
Project Number: 1
Project Date: 26/02/2020
Inspection Standard: MSCC3 Sewers & Drainage GB (SRM4 Scoring)



Table of Contents

Project Name	Project Number	Project Date
29928 Ryebank Fields	1	26/02/2020

Project Information	P-1
Section: 1; UUMH1704 > UUMH0401	1
Section: 2; UUMH0401 > UUMH9400	3



Project Information

Project Name	Project Number	Project Date
29928 Ryebank Fields	1	26/02/2020

Client

Company: BWB Consulting Limited
Street: Ryebank Road
Town or City: Chorlton

Site

Company: Ryebank Fields
Street: Ryebank Road
Town or City: Chorlton

Contractor

Company: Drain Alert
Contact: Stephen Ormisher
Department: Director
Street: Cripplegate Barn
Town or City: Cripplegate Lane
County: Houghton
Post Code: Preston PR5 0RR
Phone: 01254 851500
Fax: 01254 854004
Email: www.drain-alert.co.uk

Section Inspection - 26/02/2020

Section 1	Inspection 1	Date 26/02/20	Time 10:27	Client's Job Ref Not Specified	Weather Showers	Pre Cleaned Not Specified	PLR Not Specified
Operator L Hilton		Vehicle PK65 HFB		Camera P235 Tractor	Preset Length Not Specified	Legal Status Not Specified	Alternative ID 1

Town or Village:		Inspection Direction:	Upstream	Upstream Node:	UU MH 1704
Road:	Ryebank Road	Inspected Length:	63.60 m	Upstream Pipe Depth:	
Location:		Total Length:	63.60 m	Downstream Node:	UU MH 0401
Surface Type:		Joint Length:	0.00 m	Downstream Pipe Depth:	
Use:	Surface water	Pipe Shape:	Circular	Dia/Height:	1,275 mm
Type of Pipe:		Pipe Material:	Brick	Lining:	No Lining
Year Constructed:		Lining Material:	No Lining		
Flow Control:					
Inspection Purpose:	Sample survey to determine asset condition				

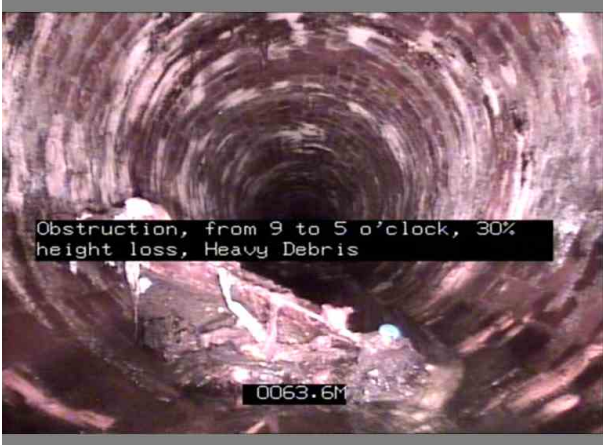
Comments:
Recommendations:

Scale:	1:554	Position [m]	Code	Observation	MPEG	Photo	Grade
Depth: m UU MH 0401							
		0.00	ST	Start of survey	00:00:23		
		63.60	OB	Obstruction, from 9 to 5 o'clock, 30% height loss: Heavy Debris	00:03:31	_2f670ca -1ec4-4ca 3-8fb1-c54	5
		63.60	FH	Finish survey: Limit of CCTV Due to Debris	00:03:37		
UU MH 1704 Depth: m							

Construction Features					Miscellaneous Features				
Structural Defects					Service & Operational Observations				
STR No. Def	STR Peak	STR Mean	STR Total	STR Grade	SER No. Def	SER Peak	SER Mean	SER Total	SER Grade
0	0.0	0.0	0.0	1.0	1	10.0	0.2	10.0	5.0

Section Pictures - 26/02/2020

Section	Inspection Direction	PLR	Client's Job Ref	Contractor's Job Ref
1	Upstream			



_2f6700ca-1ec4-4ca3-8fb1-c542ec392b47.jpg, 00:03:31,
63.60 m

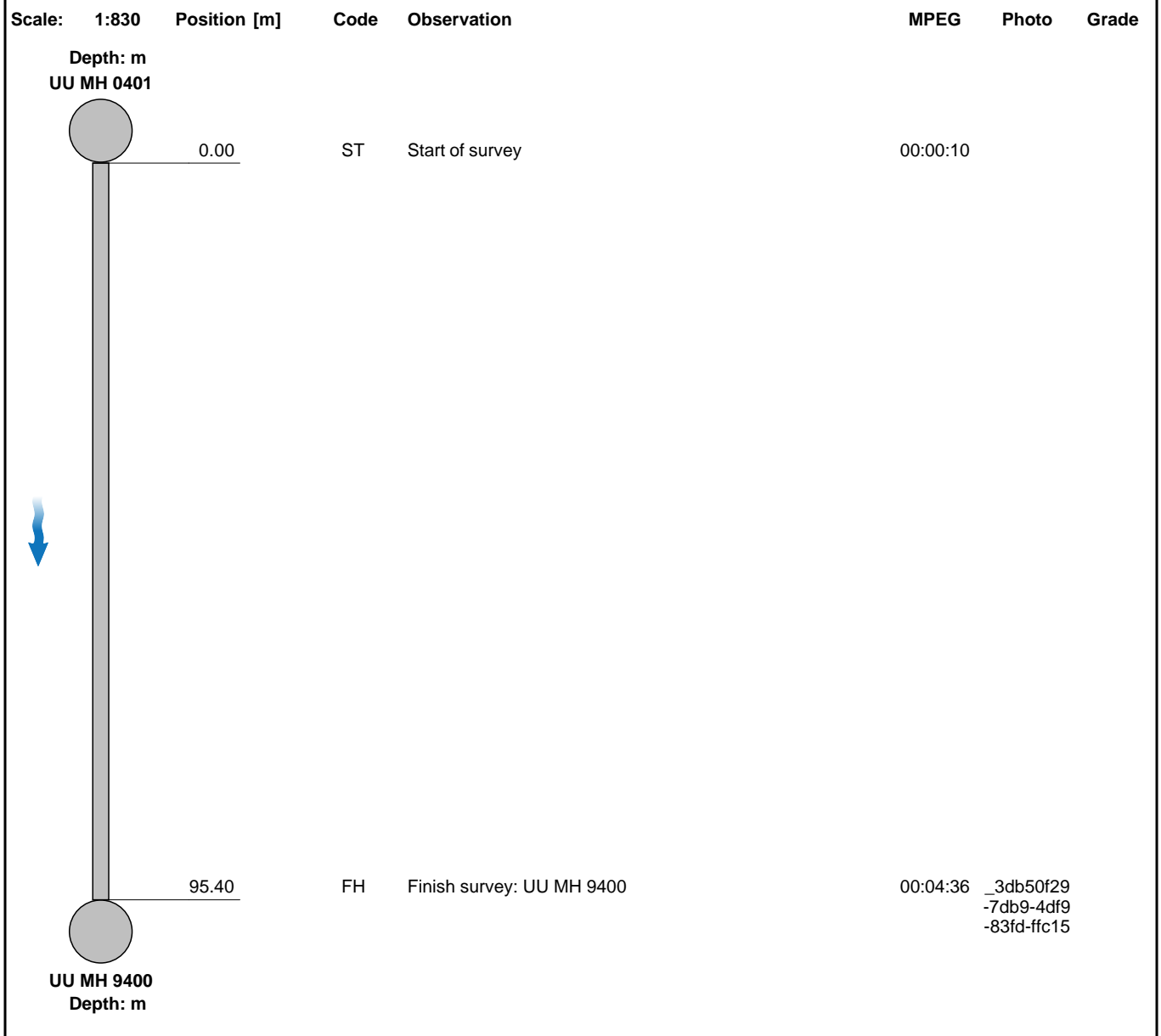
Obstruction, from 9 to 5 o'clock, 30% height loss, Heavy Debris

Section Inspection - 26/02/2020

Section 2	Inspection 1	Date 26/02/20	Time 11:27	Client's Job Ref Not Specified	Weather Showers	Pre Cleaned Not Specified	PLR Not Specified
Operator L Hilton		Vehicle PK65 HFB		Camera P235 Tractor	Preset Length Not Specified	Legal Status Not Specified	Alternative ID 2

Town or Village:	Inspection Direction:	Upstream Node:
Road: Ryebank Road	Downstream	UU MH 0401
Location:	Inspected Length: 95.40 m	Upstream Pipe Depth:
Surface Type:	Total Length: 95.40 m	Downstream Node: UU MH 9400
Use: Surface water	Joint Length: 0.00 m	Downstream Pipe Depth:
Type of Pipe:	Pipe Shape: Circular	
Year Constructed:	Dia/Height: 1,275 mm	
Flow Control:	Pipe Material: Brick	
Inspection Purpose: Sample survey to determine asset condition	Lining: No Lining	
	Lining Material: No Lining	

Comments:
 Recommendations:



Section Pictures - 26/02/2020

Section	Inspection Direction	PLR	Client's Job Ref	Contractor's Job Ref
2	Downstream			



_3db50f29-7db9-4df9-83fd-ffc15a0464f2.jpg, 00:04:36, 95.40
m
Finish survey, UU MH 9400



Manhole Record Card

Number	<input type="text" value="UU0401"/>	Date Of Survey	<input type="text" value="26/02/2020"/>
Status	<input type="text" value="PU"/>	Function	<input type="text" value="S"/>
		Type	<input type="text" value="M"/>

Cover Details:

Square Recta Double Triang Single Triangl Circular Multiple Hinged Lockable

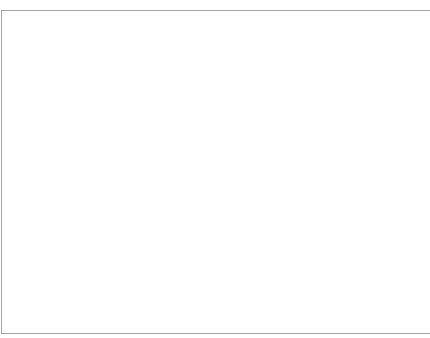
CoverLevel Cover Load Class

Cover	<input type="text" value="600"/> X <input type="text" value="600"/>	Chamber	<input type="text"/> X <input type="text" value="0"/>	EvidenceOfSurcharge	<input type="checkbox"/>
Shaft	<input type="text" value="0"/> X <input type="text" value="0"/>	ShaftDepth	<input type="text" value="0"/>	ToxicAtmosphere	<input type="checkbox"/>
Brick	<input checked="" type="checkbox"/>	Precast Concrete	<input type="checkbox"/>	PVC	<input type="checkbox"/>
Reducing Slab	<input type="checkbox"/>	Taper	<input type="checkbox"/>	Side Entry	<input type="checkbox"/>
		No.Land	<input type="text" value="0"/>	Step Irons	<input checked="" type="checkbox"/>
		Segments	<input type="checkbox"/>	No.RegCourses	<input type="text"/>
		Ladder	<input type="checkbox"/>		

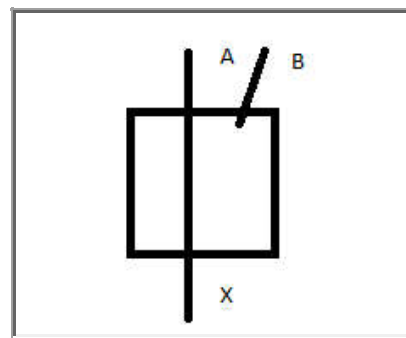
PlanPhoto



LocationPhoto



PlanofManhol



Chamber Conditions:

Cover	<input type="text" value="OK"/>	Shaft	<input type="text" value="OK"/>
Irons/Ladder	<input type="text" value="OK"/>	Chamber	<input type="text" value="OK"/>
Benching/Channel	<input type="text" value="OK"/>		

PHOTOS NOT DOWLOADING, WILL RE TAKE WHEN WE GO BACK TO SITE

Pipe	Invert L	Depth	Fr	UpstreamRe	Downstream	Pipe Sh	Size	Height	Size	Width	Pipe Material	Lining Material
A	-5.400	5.40	MH1704			C			1275		B	
B	-2.730	2.73				C			150		CI	
X	-5.400	5.40			MH9400	C			1275		B	

Disclaimer - Any dimensions and levels provided on this form should be checked before being relied upon. It is the responsibility of the customer to verify all information given with regards to the drainage prior to designing or commencing any work on site.



Manhole ID	Cover	Func	Invert	Size x	Size y	Shape	Mat	Length	Grad
4010	CO	CO	24.02	150	150	VC	10.2707	10.7677	1 in 121
4011	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 141
4012	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
4013	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
4014	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
4015	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
4016	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
4017	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
4018	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
4019	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
4020	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
4021	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
4022	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
4023	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
4024	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
4025	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
4026	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
4027	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
4028	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
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4030	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
4031	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
4032	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
4033	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
4034	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
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4039	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
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4043	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
4044	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
4045	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
4046	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
4047	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
4048	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
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4054	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
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4066	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
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4068	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
4069	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
4070	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
4071	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
4072	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
4073	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
4074	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
4075	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
4076	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
4077	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
4078	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
4079	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
4080	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
4081	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
4082	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
4083	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
4084	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
4085	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
4086	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
4087	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
4088	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
4089	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
4090	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
4091	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
4092	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
4093	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
4094	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
4095	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
4096	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
4097	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
4098	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
4099	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207
4100	CO	CO	24.02	150	150	VC	10.7677	10.7677	1 in 207

LEGEND

--- Abandoned
--- Foul
--- Surface Water
--- Combined
--- Public Sewer
--- Private Sewer
--- Section 104
--- Rising Main
--- Sludge Main
--- Overflow
--- Water Course
--- Highway Drain

All point assets follow the standard colour convention:
● red - combined
● blue - surface water
● purple - foul

● Manhole
● Head of System
● Extent of Survey
● Rodding Eye
● Inlet
● Discharge Point
● Vortex
● Penstock
● Washout Chamber
● Valve
● Air Valve
● Non Return Valve
● Soakaway
● Gully
● Cascade
● Flow Meter
● Hatch Box
● Oil Interceptor
● Summit
● Drop
● Orifice Plate

● Side Entry Manhole
● Outfall
● Screen Chamber
● Inspection Chamber
● Bifurcation Chamber
● Lamp Hole
● T Junction / Saddle
● Catchpit
● Valve Chamber
● Vent Column
● Vortex Chamber
● Penstock Chamber
● Network Storage Tank
● Sewer Overflow
● Ww Pumping Station
● Ww Treatment Works
● Septic Tank
● Control Kiosk
● Change of Characteristic

MANHOLE FUNCTION
 FO Foul
 SW Surface Water
 CO Combined
 OV Overflow

SEWER SHAPE
 CI Circular
 TR Trapezoidal
 EG Egg
 AR Arch
 OV Oval
 BA Barrel
 FT Flat Top
 HO Horseshoe
 RE Rectangular
 UN Unspecified
 SQ Square

SEWER MATERIAL
 AC Asbestos Cement
 BR Brick
 PE Polyethylene
 RP Reinforced Plastic Matrix
 CO Concrete
 CSB Concrete Segment Bolted
 CSU Concrete Segment Unbolted
 CO Concrete Box Culverted
 PSC Plastic / Steel Composite
 GRC Glass Reinforced Plastic
 DI Ductile Iron
 PVC Polyvinyl Chloride
 CI Cast Iron
 SI Spun Iron
 ST Steel
 VC Vitreous Clay
 PP Polypropylene
 PF Pitch Fibre
 MAC Masonry, Coursed
 MAR Masonry, Random
 U Unspecified

Address or Site Reference:
 Ryebank Fields,

OS sheet SJ8194NW
Number:
Scale: 1:1250
Nodes: 202
Sheet: 4 of 6
Date: 28/10/2019
Printed by: Property Searches

The position of the underground apparatus shown on this plan is approximate only and is given in accordance with the best information currently available. United Utilities Water will not accept liability for any loss or damage caused by the actual position being different from those shown.

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SEWER RECORDS



Refno	Cover	Func	Invert	Size x	Size y	Shape	Mat	Length	Grad
9165	VC	CD	22.5	1200	150	BR	93.0215	1 in 1860	
9166	VC	CD	22.5	1200	150	VC	3.938642	1 in 51	
8003	VC	CD	24.29	300	150	VC	45.80393	1 in 51	
8004	VC	CD	24.29	300	150	VC	33.8778	1 in 51	
8206	VC	CD	23.46	150	150	VC	13.15238	1 in 279	
7001	VC	CD	23.46	150	150	VC	53.00043	1 in 279	
8104	VC	CD	0	150	150	VC	7.84548	1 in 273	
8105	VC	CD	22.05	375	150	VC	76.55094	1 in 273	
4401	VC	CD	22.05	375	150	VC	76.55094	1 in 273	
0000	VC	CD	25.99	150	150	VC	37.72682	1 in 598	
0000	VC	CD	25.99	300	300	VC	71.70077	1 in 598	
8000	VC	CD	24.87	225	225	VC	25.09983	1 in 45	
8202	VC	CD	26.03	225	225	VC	25.4951	1 in 425	
8001	VC	CD	23.51	300	300	VC	59.13544	1 in 191	
8001	VC	CD	23.51	300	300	VC	59.13544	1 in 191	
8000	VC	CD	26.15	225	225	VC	63.82096	1 in 95	
6128	VC	CD	23.89	375	150	VC	2.280235	1 in 100	
8019	VC	CD	23.89	375	150	VC	74.70699	1 in 100	
8019	VC	CD	23.89	375	150	VC	3.905596	1 in 100	
8001	VC	CD	23.14	375	150	VC	51.10773	1 in 100	
8002	VC	CD	23.38	300	300	VC	24.62877	1 in 35	
8002	VC	CD	23.38	300	300	VC	6.708204	1 in 35	
8508	VC	CD	26.85	150	150	VC	2.13211	1 in 190	
8509	VC	CD	26.85	150	150	VC	1.21211	1 in 190	
8509	VC	CD	26.85	150	150	VC	6.587931	1 in 190	
8201	VC	CD	27.15	225	225	VC	6.587931	1 in 152	
0000	VC	CD	0	225	225	VC	30.41381	1 in 152	
0000	VC	CD	0	225	225	VC	51.48304	1 in 152	
0000	VC	CD	0	225	225	VC	51.48304	1 in 152	
7207	VC	CD	24.3	150	150	VC	43.56694	1 in 168	
7119	VC	CD	24.3	150	150	VC	8.002165	1 in 168	
7204	VC	CD	24.79	225	225	VC	43.01183	1 in 89	
7402	VC	CD	25.29	375	375	VC	40.78216	1 in 157	
8202	VC	CD	21.1	1972	1972	VC	88.33971	1 in 252	
6301	VC	CD	0	300	300	VC	12.18817	1 in 300	
5101	VC	SW	24.22	1200	150	BR	82.48242	1 in 300	
8202	VC	CD	22.28	375	375	VC	8.480211	1 in 22	
4505	VC	CD	25.05	375	375	VC	79.2282	1 in 334	
8502	VC	CD	25.05	225	225	VC	79.2282	1 in 334	
8502	VC	CD	25.05	225	225	VC	40.24629	1 in 149	
8502	VC	CD	25.05	225	225	VC	40.24629	1 in 149	
9208	VC	CD	25.05	225	225	VC	5.282632	1 in 149	
6122	VC	CD	23.95	375	150	VC	17.85259	1 in 353	
8201	VC	CD	23.95	375	150	VC	9.127459	1 in 353	
8314	VC	CD	22.61	600	600	VC	82.81981	1 in 157	
8117	VC	CD	24.8	150	150	VC	18.24986	1 in 167	
7108	VC	CD	24.8	150	150	VC	13.47075	1 in 167	
0400	VC	CD	24.88	375	375	VC	35.05799	1 in 167	
0400	VC	CD	24.88	375	375	VC	35.05799	1 in 167	
7203	VC	CD	24.04	100	100	VC	4.902229	1 in 167	
7103	VC	CD	24.04	100	100	VC	8.400207	1 in 167	
8017	VC	CD	24.3	100	100	VC	7.308312	1 in 167	
7205	VC	CD	25.81	100	100	VC	8.400207	1 in 167	
8007	VC	CD	25.81	100	100	VC	16.76758	1 in 128	
9101	VC	CD	25.81	300	300	VC	46.04346	1 in 128	
9101	VC	CD	25.81	300	300	VC	28.57066	1 in 128	
0103	VC	CD	25.83	300	300	VC	28.57066	1 in 62	
7201	VC	CD	24.94	225	225	VC	13.80244	1 in 107	
6101	VC	CD	25.1	300	300	VC	39.35734	1 in 252	
8101	VC	CD	25.43	300	300	VC	54.40588	1 in 85	
8101	VC	CD	25.43	300	300	VC	45.27862	1 in 85	
5301	VC	CD	25.17	150	150	VC	84.21995	1 in 241	
8108	VC	CD	24.8	150	150	VC	78.02031	1 in 157	
8500	VC	CD	24.37	225	225	VC	32.26244	1 in 157	
8500	VC	CD	24.37	225	225	VC	32.26244	1 in 157	
8000	VC	CD	24.17	375	375	VC	58.83027	1 in 294	
0200	VC	CD	26.52	300	300	VC	7.80982	1 in 255	
0200	VC	CD	26.52	300	300	VC	56.0803	1 in 255	
8007	VC	CD	26.52	300	300	VC	56.0803	1 in 255	
0308	VC	CD	0	150	150	VC	15.17119	1 in 255	
0308	VC	CD	0	150	150	VC	8.0146	1 in 255	
7117	VC	CD	23.94	150	150	VC	17.34484	1 in 211	
8102	VC	CD	23.94	150	150	VC	61.23051	1 in 211	
5502	VC	CD	23.9	300	300	VC	20.88061	1 in 29	
5502	VC	CD	23.9	300	300	VC	20.88061	1 in 29	
7003	VC	SW	0	1200	150	BR	127.1668	1 in 29	
0312	VC	CD	0	150	150	VC	6.28277	1 in 155	
8102	VC	CD	26.52	225	225	VC	14.73944	1 in 155	
0102	VC	CD	26.52	225	225	VC	50.09982	1 in 155	
4303	VC	CD	25.51	150	150	VC	67.77066	1 in 198	
9303	VC	CD	24.28	375	375	VC	45.70626	1 in 452	
9401	VC	CD	24.8	375	375	VC	64.626	1 in 166	
9401	VC	CD	24.8	375	375	VC	54.626	1 in 166	
7004	VC	SW	0	1200	150	BR	39.12702	1 in 64	
9304	VC	CD	24.87	300	300	VC	36.87424	1 in 64	
8000	VC	CD	23.25	375	375	VC	55.88994	1 in 622	
8000	VC	CD	24.44	375	375	VC	54.66655	1 in 622	
8117	VC	CD	24.8	150	150	VC	8.81529	1 in 419	
8117	VC	CD	24.8	150	150	VC	8.81529	1 in 419	
8401	VC	CD	24.82	150	150	VC	74.27851	1 in 419	
8108	VC	CD	24.8	150	150	VC	86.05881	1 in 419	
8202	VC	CD	23.2	300	300	VC	15.3287	1 in 520	
8202	VC	CD	23.2	300	300	VC	19.74793	1 in 520	
8207	VC	CD	23.2	300	300	VC	15.33975	1 in 520	
8414	VC	CD	22.96	375	375	VC	39.4206	1 in 520	
9102	VC	CD	25.71	225	225	VC	36.84972	1 in 100	
9102	VC	CD	25.71	225	225	VC	36.84972	1 in 100	
7100	VC	CD	23.64	300	300	VC	50.22277	1 in 351	
8201	VC	CD	24.78	225	225	VC	52.35456	1 in 128	
8201	VC	CD	24.78	225	225	VC	52.35456	1 in 128	
7118	VC	CD	24.78	150	150	VC	12.33419	1 in 100	
8212	VC	CD	22.34	150	150	VC	7.30716	1 in 2084	
8203	VC	SW	26.02	1200	150	BR	145.9875	1 in 40	
5102	VC	CD	26.02	150	150	VC	4.72751	1 in 40	
7401	VC	CD	26.02	150	150	VC	59.5063	1 in 40	
7401	VC	CD	26.02	150	150	VC	46.30114	1 in 208	
7004	VC	CD	25.23	225	225	VC	46.30114	1 in 208	
8201	VC	CD	21.45	1372	1372	VC	129.784	1 in 322	
8001	VC	CD	23.17	300	300	VC	73.46227	1 in 4830	
0401	VC	SW	22.54	1275	1275	BR	48.30114	1 in 4830	
0401	VC	SW	22.54	1275	1275	BR	48.30114	1 in 4830	
0401	VC	SW	22.54	1275	1275	BR	48.30114	1 in 4830	
0401	VC	SW	22.54	1275	1275	BR	48.30114	1 in 4830	
8302	VC	CD	22.43	1200	150	BR	152.8226	1 in 1870	
8302	VC	CD	22.43	1200	150	BR	152.8226	1 in 1870	
8206	VC	CD	26.08	150	150	VC	9.937974	1 in 89	
7402	VC	CD	26.08	150	150	VC	4.12108	1 in 89	
7402	VC	CD	26.08	150	150	VC	4.12108	1 in 89	
8203	VC	CD	24.46	225	225	VC	108.6304	1 in 205	
8301	VC	CD	26.08	300	300	VC	95.18929	1 in 154	
8301	VC	CD	26.08	300	300	VC	95.18929	1 in 154	
0301	VC	CD	0	300	300	VC	89.10687	1 in 252	
5301	VC	CD	22.48	375	375	VC	38.40055	1 in 202	
8501	VC	CD	22.44	375	375	VC	36.40055	1 in 202	

LEGEND

Abandoned Foul Surface Water Combined Public Sewer
 Private Sewer Section 104
 Rising Main Sludge Main
 Overflow Water Course
 Highway Drain

All point assets follow the standard colour convention:
 red - combined blue - surface water
 brown - foul purple - overflow

- Manhole
- Head of System
- Extent of Survey
- Rodding Eye
- Inlet
- Discharge Point
- Vortex
- Penstock
- Washout Chamber
- Valve
- Air Valve
- Non Return Valve
- Soakaway
- Gully
- Cascade
- Flow Meter
- Hatch Box
- Oil Interceptor
- Summit
- Drop Shaft
- Orifice Plate
- Side Entry Manhole
- Outfall
- Screen Chamber
- Inspection Chamber
- Bifurcation Chamber
- Lamp Hole
- T Junction / Saddle
- Catchpit
- Valve Chamber
- Vent Column
- Vortex Chamber
- Penstock Chamber
- Network Storage Tank
- Sewer Overflow
- Ww Treatment Works
- Ww Pumping Station
- Septic Tank
- Control Kiosk
- Change of Characteristic

MANHOLE FUNCTION

FO Foul
 SW Surface Water
 CO Combined
 OV Overflow

SEWER SHAPE

CI Circular TR Trapezoidal
 EG Egg AR Arch
 OV Oval BA Barrel
 FT Flat Top HO HorseShoe
 RE Rectangular UN Unspecified
 SQ Square

SEWER MATERIAL

AC Albestos Cement
 BR Brick
 PE Polyethylene
 RP Reinforced Plastic Matrix
 CO Concrete
 CSB Concrete Segment Bolted
 CSU Concrete Segment Unbolted
 CC Concrete Box Culvert
 PSC Plastic / Steel Composite
 GRC Glass Reinforced Plastic
 DI Ductile Iron
 PVC Polyvinyl Chloride
 CI Cast Iron
 SI Spun Iron
 ST Steel
 VC Vitified Clay
 PP Polypropylene
 PF Pitch Fibre
 MAC Masonry, Coursed
 MAR Masonry, Random
 U Unspecified

— LINES SURVEYED
 ● MH REF'S

The position of the underground apparatus shown on this plan is approximate only and is given in accordance with the best information currently available. United Utilities Water will not accept liability for any loss or damage caused by the actual position being different from those shown.

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Address or Site Reference:
 Ryebank Fields,

OS sheet SJ8094SE
Number:
Scale: 1:1250 **Date:** 28/10/2019
Nodes: 135
Sheet: 1 of 6

Printed by: Property Searches



APPENDIX 7: Micro Drainage Calculations

BWB Consulting Ltd		Page 1
5th Floor, Waterfront House 35 Station Street Nottingham, NG2 3DQ		
Date 03/03/2020 08:23 File	Designed by keith.alger Checked by	
XP Solutions		Source Control 2018.1.1

ICP SUDS Mean Annual Flood

Input

Return Period (years)	2	Soil	0.300
Area (ha)	1.000	Urban	0.000
SAAR (mm)	851	Region Number	Region 10

Results 1/s

QBAR Rural 2.3
QBAR Urban 2.3

Q2 years 2.1

Q1 year 2.0
Q30 years 3.9
Q100 years 4.8

Summary of Results for 100 year Return Period (+40%)


Storm Event	Max Level (m)	Max Depth (m)	Max Control (l/s)	Max Volume (m ³)	Status
15 min Summer	99.431	0.431	4.5	470.8	O K
30 min Summer	99.488	0.488	4.5	537.5	O K
60 min Summer	99.549	0.549	4.5	611.2	O K
120 min Summer	99.614	0.614	4.5	690.4	O K
180 min Summer	99.652	0.652	4.5	737.4	O K
240 min Summer	99.678	0.678	4.5	769.8	O K
360 min Summer	99.711	0.711	4.5	812.0	Flood Risk
480 min Summer	99.731	0.731	4.5	837.6	Flood Risk
600 min Summer	99.744	0.744	4.5	853.5	Flood Risk
720 min Summer	99.751	0.751	4.5	863.3	Flood Risk
960 min Summer	99.763	0.763	4.5	878.9	Flood Risk
1440 min Summer	99.764	0.764	4.5	879.7	Flood Risk
2160 min Summer	99.743	0.743	4.5	852.2	Flood Risk
2880 min Summer	99.721	0.721	4.5	823.9	Flood Risk
4320 min Summer	99.673	0.673	4.5	763.4	O K
5760 min Summer	99.625	0.625	4.5	704.3	O K
7200 min Summer	99.573	0.573	4.5	639.2	O K
8640 min Summer	99.523	0.523	4.5	579.1	O K
10080 min Summer	99.476	0.476	4.5	523.8	O K
15 min Winter	99.480	0.480	4.5	528.0	O K
30 min Winter	99.543	0.543	4.5	603.3	O K

Storm Event	Rain (mm/hr)	Flooded Volume (m ³)	Discharge Volume (m ³)	Time-Peak (mins)
15 min Summer	200.129	0.0	379.8	27
30 min Summer	114.737	0.0	379.6	41
60 min Summer	65.780	0.0	626.0	72
120 min Summer	37.713	0.0	703.0	130
180 min Summer	27.237	0.0	719.3	190
240 min Summer	21.621	0.0	716.8	250
360 min Summer	15.615	0.0	707.3	368
480 min Summer	12.396	0.0	698.2	488
600 min Summer	10.363	0.0	690.3	606
720 min Summer	8.952	0.0	683.3	726
960 min Summer	7.164	0.0	670.7	964
1440 min Summer	5.233	0.0	650.3	1442
2160 min Summer	3.823	0.0	1310.3	1868
2880 min Summer	3.059	0.0	1325.0	2256
4320 min Summer	2.223	0.0	1219.5	3036
5760 min Summer	1.772	0.0	1619.8	3880
7200 min Summer	1.486	0.0	1698.5	4680
8640 min Summer	1.288	0.0	1765.5	5440
10080 min Summer	1.140	0.0	1824.3	6160
15 min Winter	200.129	0.0	380.7	27
30 min Winter	114.737	0.0	374.7	41

Summary of Results for 100 year Return Period (+40%)

Storm Event	Max Level (m)	Max Depth (m)	Max Control (l/s)	Max Volume (m ³)	Status
60 min Winter	99.611	0.611	4.5	686.8	O K
120 min Winter	99.684	0.684	4.5	776.9	O K
180 min Winter	99.726	0.726	4.5	830.9	Flood Risk
240 min Winter	99.755	0.755	4.5	868.5	Flood Risk
360 min Winter	99.794	0.794	4.5	918.6	Flood Risk
480 min Winter	99.818	0.818	4.5	950.1	Flood Risk
600 min Winter	99.834	0.834	4.5	970.9	Flood Risk
720 min Winter	99.844	0.844	4.5	984.5	Flood Risk
960 min Winter	99.862	0.862	4.5	1007.6	Flood Risk
1440 min Winter	99.871	0.871	4.5	1019.4	Flood Risk
2160 min Winter	99.854	0.854	4.5	997.2	Flood Risk
2880 min Winter	99.824	0.824	4.5	957.8	Flood Risk
4320 min Winter	99.764	0.764	4.5	879.9	Flood Risk
5760 min Winter	99.700	0.700	4.5	798.2	Flood Risk
7200 min Winter	99.631	0.631	4.5	711.6	O K
8640 min Winter	99.549	0.549	4.5	610.4	O K
10080 min Winter	99.474	0.474	4.5	521.1	O K

Storm Event	Rain (mm/hr)	Flooded Volume (m ³)	Discharge Volume (m ³)	Time-Peak (mins)
60 min Winter	65.780	0.0	694.1	70
120 min Winter	37.713	0.0	719.6	128
180 min Winter	27.237	0.0	713.1	188
240 min Winter	21.621	0.0	706.2	246
360 min Winter	15.615	0.0	695.5	362
480 min Winter	12.396	0.0	687.8	480
600 min Winter	10.363	0.0	681.9	596
720 min Winter	8.952	0.0	677.1	712
960 min Winter	7.164	0.0	670.1	942
1440 min Winter	5.233	0.0	664.5	1394
2160 min Winter	3.823	0.0	1378.6	2040
2880 min Winter	3.059	0.0	1344.5	2368
4320 min Winter	2.223	0.0	1250.9	3252
5760 min Winter	1.772	0.0	1814.6	4208
7200 min Winter	1.486	0.0	1902.1	5120
8640 min Winter	1.288	0.0	1977.5	5888
10080 min Winter	1.140	0.0	2043.4	6656

BWB Consulting Ltd		Page 3
4th Floor Carvers Warehouse 77 Dale Street Manchester M1 2HG		
Date 04/03/2020 15:08 File Catchment 1 Storage.SRCX	Designed by Lucy.Reeves Checked by	
Micro Drainage		Source Control 2018.1.1


Rainfall Details

Rainfall Model	FEH
Return Period (years)	100
FEH Rainfall Version	1999
Site Location	GB 380950 394650 SJ 80950 94650
C (1km)	-0.026
D1 (1km)	0.317
D2 (1km)	0.345
D3 (1km)	0.332
E (1km)	0.303
F (1km)	2.456
Summer Storms	Yes
Winter Storms	Yes
Cv (Summer)	0.750
Cv (Winter)	0.840
Shortest Storm (mins)	15
Longest Storm (mins)	10080
Climate Change %	+40

Time Area Diagram

Total Area (ha) 1.270

Time (mins)	Area (ha)	Time (mins)	Area (ha)	Time (mins)	Area (ha)
From:	To:	From:	To:	From:	To:
0	4	4	8	8	12
	0.423		0.423		0.423

BWB Consulting Ltd		Page 4
4th Floor Carvers Warehouse 77 Dale Street Manchester M1 2HG		
Date 04/03/2020 15:08 File Catchment 1 Storage.SRCX	Designed by Lucy.Reeves Checked by	
Micro Drainage		Source Control 2018.1.1

Model Details

Storage is Online Cover Level (m) 100.000

Tank or Pond Structure

Invert Level (m) 99.000

Depth (m)	Area (m ²)	Depth (m)	Area (m ²)
0.000	1016.3	1.000	1383.6

Hydro-Brake® Optimum Outflow Control

Unit Reference	MD-SHE-0100-4500-1050-4500
Design Head (m)	1.050
Design Flow (l/s)	4.5
Flush-Flo™	Calculated
Objective	Minimise upstream storage
Application	Surface
Sump Available	Yes
Diameter (mm)	100
Invert Level (m)	98.950
Minimum Outlet Pipe Diameter (mm)	150
Suggested Manhole Diameter (mm)	1200

Control Points	Head (m)	Flow (l/s)
Design Point (Calculated)	1.050	4.5
Flush-Flo™	0.310	4.5
Kick-Flo®	0.661	3.6
Mean Flow over Head Range	-	3.9

The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated

Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)
0.100	3.3	1.200	4.8	3.000	7.3	7.000	11.0
0.200	4.4	1.400	5.1	3.500	7.9	7.500	11.3
0.300	4.5	1.600	5.5	4.000	8.4	8.000	11.7
0.400	4.4	1.800	5.8	4.500	8.9	8.500	12.0
0.500	4.3	2.000	6.1	5.000	9.4	9.000	12.4
0.600	4.0	2.200	6.4	5.500	9.8	9.500	12.7
0.800	4.0	2.400	6.6	6.000	10.2		
1.000	4.4	2.600	6.9	6.500	10.6		

Summary of Results for 100 year Return Period (+40%)

Storm Event	Max Level (m)	Max Depth (m)	Max Control (l/s)	Max Volume (m ³)	Status
15 min Summer	99.441	0.441	3.0	251.2	O K
30 min Summer	99.496	0.496	3.0	286.5	O K
60 min Summer	99.555	0.555	3.0	325.0	O K
120 min Summer	99.616	0.616	3.0	365.6	O K
180 min Summer	99.651	0.651	3.0	388.9	O K
240 min Summer	99.673	0.673	3.0	404.4	O K
360 min Summer	99.701	0.701	3.0	423.3	Flood Risk
480 min Summer	99.715	0.715	3.0	433.5	Flood Risk
600 min Summer	99.722	0.722	3.0	438.6	Flood Risk
720 min Summer	99.725	0.725	3.0	440.4	Flood Risk
960 min Summer	99.727	0.727	3.0	442.3	Flood Risk
1440 min Summer	99.712	0.712	3.0	431.5	Flood Risk
2160 min Summer	99.683	0.683	3.0	411.4	O K
2880 min Summer	99.656	0.656	3.0	392.2	O K
4320 min Summer	99.593	0.593	3.0	349.8	O K
5760 min Summer	99.529	0.529	3.0	307.9	O K
7200 min Summer	99.471	0.471	3.0	270.2	O K
8640 min Summer	99.417	0.417	3.0	236.3	O K
10080 min Summer	99.367	0.367	3.0	205.5	O K
15 min Winter	99.489	0.489	3.0	281.8	O K
30 min Winter	99.550	0.550	3.0	321.7	O K

Storm Event	Rain (mm/hr)	Flooded Volume (m ³)	Discharge Volume (m ³)	Time-Peak (mins)
15 min Summer	200.129	0.0	243.7	27
30 min Summer	114.737	0.0	250.9	41
60 min Summer	65.780	0.0	335.2	70
120 min Summer	37.713	0.0	384.5	130
180 min Summer	27.237	0.0	416.4	190
240 min Summer	21.621	0.0	440.7	248
360 min Summer	15.615	0.0	466.9	368
480 min Summer	12.396	0.0	470.4	486
600 min Summer	10.363	0.0	468.7	606
720 min Summer	8.952	0.0	465.7	724
960 min Summer	7.164	0.0	457.9	962
1440 min Summer	5.233	0.0	440.9	1368
2160 min Summer	3.823	0.0	701.6	1712
2880 min Summer	3.059	0.0	748.6	2104
4320 min Summer	2.223	0.0	811.6	2904
5760 min Summer	1.772	0.0	867.2	3688
7200 min Summer	1.486	0.0	909.7	4464
8640 min Summer	1.288	0.0	945.4	5192
10080 min Summer	1.140	0.0	976.8	5952
15 min Winter	200.129	0.0	250.7	27
30 min Winter	114.737	0.0	250.4	41

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Source Control 2018.1.1

Summary of Results for 100 year Return Period (+40%)

Storm Event	Max Level (m)	Max Depth (m)	Max Control (l/s)	Max Volume (m ³)	Status
60 min Winter	99.616	0.616	3.0	365.6	O K
120 min Winter	99.684	0.684	3.0	412.0	O K
180 min Winter	99.723	0.723	3.0	439.1	Flood Risk
240 min Winter	99.749	0.749	3.0	457.4	Flood Risk
360 min Winter	99.781	0.781	3.0	480.6	Flood Risk
480 min Winter	99.800	0.800	3.0	493.9	Flood Risk
600 min Winter	99.810	0.810	3.0	501.5	Flood Risk
720 min Winter	99.815	0.815	3.0	505.4	Flood Risk
960 min Winter	99.823	0.823	3.0	511.2	Flood Risk
1440 min Winter	99.816	0.816	3.0	505.7	Flood Risk
2160 min Winter	99.780	0.780	3.0	479.8	Flood Risk
2880 min Winter	99.747	0.747	3.0	455.8	Flood Risk
4320 min Winter	99.668	0.668	3.0	400.5	O K
5760 min Winter	99.573	0.573	3.0	336.8	O K
7200 min Winter	99.478	0.478	3.0	275.1	O K
8640 min Winter	99.394	0.394	3.0	222.5	O K
10080 min Winter	99.320	0.320	3.0	177.4	O K

Storm Event	Rain (mm/hr)	Flooded Volume (m ³)	Discharge Volume (m ³)	Time-Peak (mins)
60 min Winter	65.780	0.0	375.4	70
120 min Winter	37.713	0.0	430.6	128
180 min Winter	27.237	0.0	462.9	186
240 min Winter	21.621	0.0	471.8	244
360 min Winter	15.615	0.0	471.9	362
480 min Winter	12.396	0.0	468.6	478
600 min Winter	10.363	0.0	464.8	594
720 min Winter	8.952	0.0	461.1	708
960 min Winter	7.164	0.0	454.0	936
1440 min Winter	5.233	0.0	441.4	1378
2160 min Winter	3.823	0.0	785.9	1932
2880 min Winter	3.059	0.0	838.4	2228
4320 min Winter	2.223	0.0	831.9	3160
5760 min Winter	1.772	0.0	971.5	4040
7200 min Winter	1.486	0.0	1018.6	4768
8640 min Winter	1.288	0.0	1058.9	5536
10080 min Winter	1.140	0.0	1094.4	6256

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Rainfall Details

Rainfall Model	FEH
Return Period (years)	100
FEH Rainfall Version	1999
Site Location	GB 380950 394650 SJ 80950 94650
C (1km)	-0.026
D1 (1km)	0.317
D2 (1km)	0.345
D3 (1km)	0.332
E (1km)	0.303
F (1km)	2.456
Summer Storms	Yes
Winter Storms	Yes
Cv (Summer)	0.750
Cv (Winter)	0.840
Shortest Storm (mins)	15
Longest Storm (mins)	10080
Climate Change %	+40

Time Area Diagram

Total Area (ha) 0.680

Time (mins)	Area	Time (mins)	Area	Time (mins)	Area
From:	To:	From:	To:	From:	To:
	(ha)		(ha)		(ha)
0	4	4	8	8	12
	0.227		0.227		0.227

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Model Details

Storage is Online Cover Level (m) 100.000

Tank or Pond Structure

Invert Level (m) 99.000

Depth (m)	Area (m ²)	Depth (m)	Area (m ²)
0.000	515.1	1.000	784.8

Hydro-Brake® Optimum Outflow Control

Unit Reference	MD-SHE-0082-3000-1050-3000
Design Head (m)	1.050
Design Flow (l/s)	3.0
Flush-Flo™	Calculated
Objective	Minimise upstream storage
Application	Surface
Sump Available	Yes
Diameter (mm)	82
Invert Level (m)	98.950
Minimum Outlet Pipe Diameter (mm)	100
Suggested Manhole Diameter (mm)	1200

Control Points	Head (m)	Flow (l/s)
Design Point (Calculated)	1.050	3.0
Flush-Flo™	0.320	3.0
Kick-Flo®	0.658	2.4
Mean Flow over Head Range	-	2.6

The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated

Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)
0.100	2.4	1.200	3.2	3.000	4.9	7.000	7.3
0.200	2.9	1.400	3.4	3.500	5.2	7.500	7.5
0.300	3.0	1.600	3.6	4.000	5.6	8.000	7.7
0.400	3.0	1.800	3.8	4.500	5.9	8.500	8.0
0.500	2.9	2.000	4.0	5.000	6.2	9.000	8.2
0.600	2.7	2.200	4.2	5.500	6.5	9.500	8.4
0.800	2.6	2.400	4.4	6.000	6.8		
1.000	2.9	2.600	4.6	6.500	7.0		

APPENDIX 8: Conceptual Drainage Strategy

UNITED UTILITIES MANHOLE REFERENCE: 1704
 LOCATION OF MANHOLE IS APPROXIMATE BASED ON
 UNITED UTILITIES SEWER RECORDS AND SITE VISIT

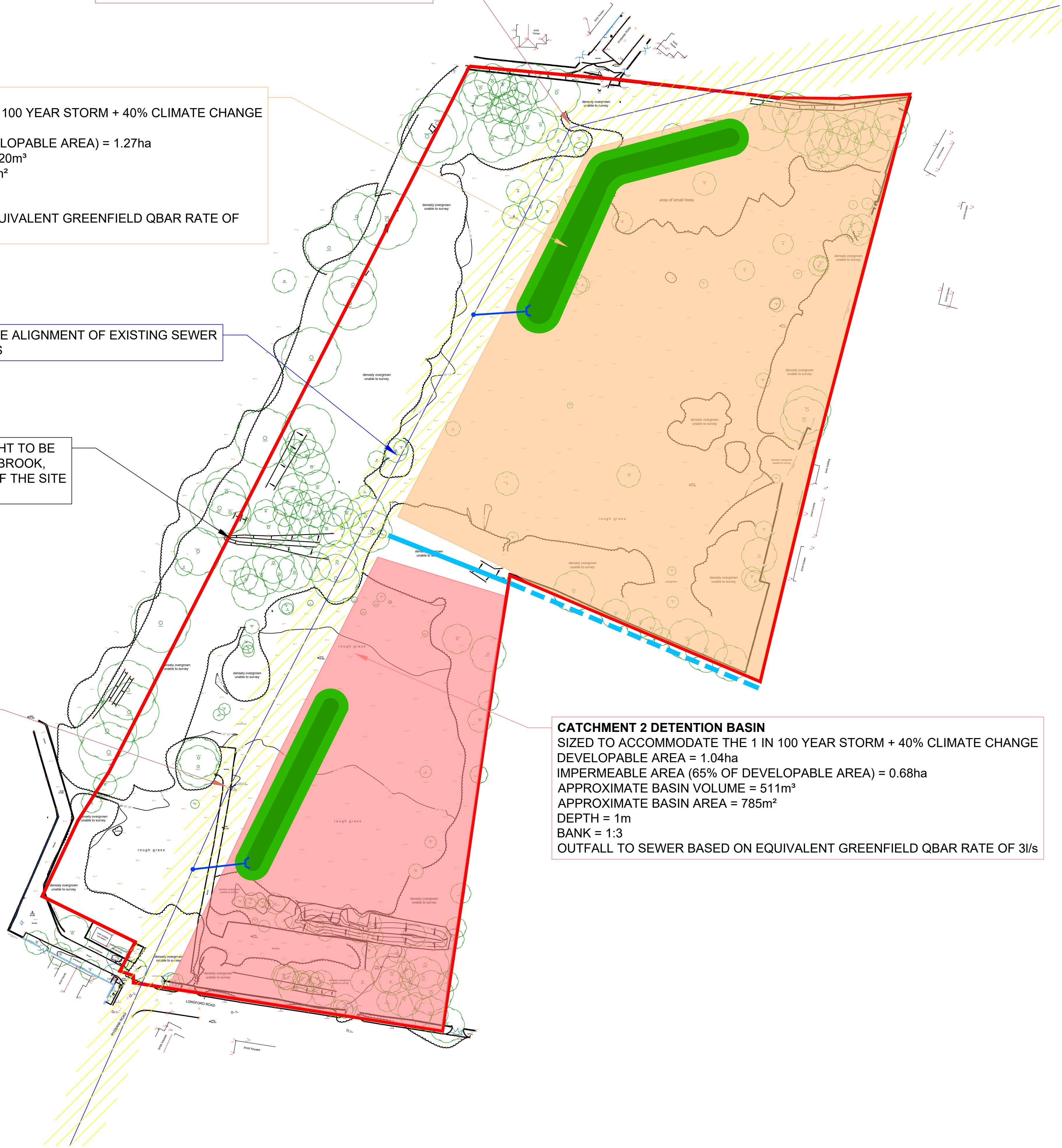
CATCHMENT 1 DETENTION BASIN
 SIZED TO ACCOMMODATE THE 1 IN 100 YEAR STORM + 40% CLIMATE CHANGE
 DEVELOPABLE AREA = 1.95ha
 IMPERMEABLE AREA (65% OF DEVELOPABLE AREA) = 1.27ha
 APPROXIMATE BASIN VOLUME = 1020m³
 APPROXIMATE BASIN AREA = 1384m²
 DEPTH = 1m
 BANK = 1:3
 OUTFALL TO SEWER BASED ON EQUIVALENT GREENFIELD QBAR RATE OF
 4.5l/s

APPROXIMATE ALIGNMENT OF EXISTING SEWER
 AND IP MAINS

LOCATION OF CULVERT THOUGHT TO BE
 ASSOCIATED WITH LONGFORD BROOK,
 ALIGNMENT OF TO THE WEST OF THE SITE
 IS NOT KNOWN

UNITED UTILITIES MANHOLE
 REFERENCE: 0401

CATCHMENT 2 DETENTION BASIN
 SIZED TO ACCOMMODATE THE 1 IN 100 YEAR STORM + 40% CLIMATE CHANGE
 DEVELOPABLE AREA = 1.04ha
 IMPERMEABLE AREA (65% OF DEVELOPABLE AREA) = 0.68ha
 APPROXIMATE BASIN VOLUME = 511m³
 APPROXIMATE BASIN AREA = 785m²
 DEPTH = 1m
 BANK = 1:3
 OUTFALL TO SEWER BASED ON EQUIVALENT GREENFIELD QBAR RATE OF 3l/s



- Notes**
- Do not scale this drawing. All dimensions must be checked/ verified on site. If in doubt ask.
 - This drawing is to be read in conjunction with all relevant architects, engineers and specialists drawings and specifications.
 - All dimensions in millimetres unless noted otherwise. All levels in metres unless noted otherwise.
 - Any discrepancies noted on site are to be reported to the engineer immediately.
 - Do not construct based on this drawing.
 - Location of existing surface water sewer/IP Mains is approximate and taken from Drain Alert site investigations and Fuel Solutions UK Limited Utility Assessment Report.
 - Location of the Nico Ditch is approximate.
 - Topographical Survey undertaken by CT Surveys.
 - All SuDS features are indicative and subject to further development of a masterplan, planning and detailed design.
 - Outfall to the existing surface water sewer subject to confirmation from United Utilities.

- Legend**
- Indicative Site Boundary
 - Nico Ditch
 - Existing Surface Water Sewer/IP Mains
 - Existing Manhole
 - 16m Easement Associated with Surface Water Sewer
 - Catchment 1
 - Catchment 2
 - Detention Basin
 - Headwall and Pipe
 - Proposed Manhole

P01	05.03.20	Preliminary Issue	LR	KA
Rev	Date	Details of issue / revision	Drw	Rev

Issues & Revisions

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Client
MANCHESTER METROPOLITAN UNIVERSITY

Project Title
RYEBANK ROAD, CHORLTON

Drawing Title
CONCEPTUAL SURFACE WATER DRAINAGE STRATEGY

Drawn:	L. Reeves	Reviewed:	K. Alger
BWB Ref:	MCW2136	Date:	05.03.20
Scale:	A1:	NTS	
Drawing Status			
PRELIMINARY			
Project - Originator - Zone - Level - Type - Role - Number	Status	Rev	
MMU-BWB-ZZ-XX-SK-CD-0001	S2	P01	

APPENDIX 9: Pre-Development Enquiry

Lucy Reeves

From: Wastewater Developer Services <WastewaterDeveloperServices@uuplc.co.uk>
Sent: 07 November 2019 13:44
To: Lucy Reeves
Cc: Wastewater Developer Services
Subject: RE: Wastewater Pre-Development Enquiry: Ryebank Road, Chorlton (MCW2136) 4200028845

Good Afternoon

We have carried out an assessment of your application which is based on the information provided; this pre development advice will be valid for 12 months

Foul

Foul will be allowed to drain to the public combined sewer network road at an unrestricted rate. Our preferred point of connection would be to the 225mm Combined sewer in Rye Bank Road, to the North of the site.

Surface Water

Surface water from this site should drain to either soak away or directly to watercourse. Discharge rates and consents must be discussed and agreed with all interested parties.

Connection Application

Although we may discuss and agree discharge points & rates in principle, please be aware that you will have to apply for a formal sewer connection. This is so that we can assess the method of construction, Health & Safety requirements and to ultimately inspect the connection when it is made. Details of the application process and the form itself can be obtained from our website by following the link below

<http://www.unitedutilities.com/connecting-public-sewer.aspx>

Sewer Adoption Agreement

You may wish to offer the proposed new sewers for adoption. United Utilities assess adoption application based on Sewers adoption 6th Edition and for any pumping stations our company addenda document. Please refer to link below to obtain further guidance and application pack:

<http://www.unitedutilities.com/sewer-adoption.aspx>

Existing Sewers Crossing the Site

A public sewer crosses this site and we will require unrestricted access to the sewer for maintenance purposes, we would ask that you maintain a minimum clearance of 6m, which is measured 3m from the centre line of the pipe. If you cannot achieve this then you may wish to consider diverting the public sewer.

Please refer to the link below to obtain full details of the processes involved in sewer diversion.

<http://www.unitedutilities.com/sewer-diversion.aspx>

Please be aware that on site drainage must be designed in accordance with Building Regulations, National Planning Policy, and local flood authority guidelines, we would recommend that you speak and make suitable agreements with the relevant statutory bodies.

Please note, if you intend to put forward your wastewater assets for adoption by United Utilities, the proposed detail design will be subject to a technical appraisal by an Adoption Engineer as we need to be sure that the proposals meets the requirements of Sewers for adoption and United Utilities Asset Standards. The proposed design should give consideration to long term operability and give United Utilities a cost effective proposal for the life of the assets. Therefore, further to this enquiry should you wish to progress a Section 104 agreement, we strongly recommend that no construction commences until the detailed drainage design, submitted as part of the Section 104 agreement, has been assessed and accepted in writing by United Utilities. Any works carried out prior to the technical assessment being approved is done entirely at the developers own risk and could be subject to change.

Regards

Matthew Dodd
Assistant Developer Engineer
Developer Services and Planning
Network Delivery
United Utilities
T: 01925 679369 (internal 79369)
unitedutilities.com

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